

GSA Hosts First Leadership Weekend, p.18

# GSA TODAY

VOL. 18, No. 8

A PUBLICATION OF THE GEOLOGICAL SOCIETY OF AMERICA

AUGUST 2008

Is there enough sand?

Evaluating the fate  
of Grand Canyon  
sandbars

***Inside:***

Panel Seeks Input on GSA Position Statement—Draft, p. 22

Overarching 2008 Joint Annual Meeting Themes, p. 31

Volume 18  
Issue 8  
August 2008

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# Is there enough sand?

## Evaluating the fate of Grand Canyon sandbars

**Scott A. Wright\***, U.S. Geological Survey, California Water Science Center, 6000 J Street, Sacramento, Calif., 95819, USA; **John C. Schmidt\***, Department of Watershed Sciences, Utah State University, Logan, Utah 84322, USA; **Theodore S. Melis\***, **David J. Topping\***, U.S. Geological Survey, Grand Canyon Monitoring and Research Center, 2255 N. Gemini Drive, Flagstaff, Ariz. 86001, USA; **David M. Rubin\***, U.S. Geological Survey, Pacific Science Center, 400 Natural Bridge Drive, Santa Cruz, Calif. 95060, USA.

### ABSTRACT

Large dams have the potential to dramatically alter the flow regime, geomorphology, and aquatic ecosystem of downstream river reaches. Development of flow release regimes in order to meet multiple objectives is a challenge facing dam operators, resource managers, and scientists. Herein, we review previous work and present new analyses related to the effects of Glen Canyon Dam on the downstream reach of the Colorado River in Marble and Grand Canyons. The dam traps the entire incoming sediment load in Lake Powell and modulates the hydrologic regime by, for example, eliminating spring snowmelt floods, resulting in changes in the geomorphology of the river downstream. The primary geomorphic impact has been the erosion of sandbars along the banks of the river. Recognition of this impact has led to many scientific studies and a variety of experimental operations of Glen Canyon Dam with the goal of rebuilding the eroding sandbars. These efforts have thus far been generally unsuccessful and the question remains as to whether or not the dam can be operated such that sandbars can be rebuilt and maintained over extended periods with the existing sediment supply. We attempt to answer this question by evaluating a dam operation that may be considered a “best-case scenario” for rebuilding and maintaining eroded sandbars. Our analysis suggests that this best-case scenario may indeed have viability for rebuilding sandbars, and that the initial rate at which sandbars could be rebuilt is comparable to the rate at which sandbars have been eroded since dam construction. The question remains open as to the viability of operations that deviate from the best-case scenario that we have defined.

### INTRODUCTION

Large dams have the potential to profoundly transform downstream riverine hydrology, geomorphology, and ecosystem function (Nilsson et al., 2005; Collier et al., 2000; Williams and Wolman, 1984; Syvitski et al., 2005). Flood control and elimination of the upstream sediment supply perturb the downstream sediment balance (Grant et al. 2003; Schmidt and Wilcock, 2008). Immediately below a large dam, sediment

deficit conditions exist because the transport capacity of the river exceeds the supply. In such a situation, fine sediment is evacuated from the channel, and the bed may become incised, potentially causing disconnection with the pre-dam floodplain and changing the distribution and availability of aquatic habitats. Sediment evacuation caused by deficit conditions has been described on many large rivers, including the Rio Grande below Elephant Butte Dam (Stevens, 1938), and the Colorado River below Glen Canyon Dam (Grams et al., 2007) and Hoover Dam (Borland and Miller, 1960). At some point farther downstream, sediment mass balance conditions may shift to surplus if there is sufficient resupply of sediment by tributaries. An extreme example of surplus is that of the Rio Grande near Presidio, Texas (Everitt, 1993). The location of the transition from deficit to surplus depends on the rate that downstream tributaries supply sediment as well as the flow regime released from the dam, which controls the transport capacity of the post-dam river. Understanding the relationship between downstream sediment supply and transport capacity is essential for resource management downstream from dams where aquatic habitat is linked to river morphology.

One notable effort to understand these relationships has been ongoing for several decades on the Colorado River in Glen and Grand Canyons downstream from Glen Canyon Dam (Fig. 1). The completion of Glen Canyon Dam in 1963 substantially reduced the downstream sediment supply by trapping in Lake Powell reservoir (Topping et al., 2000a, 2000b). In addition, flow regulation by the dam has eliminated the large annual snowmelt floods while increasing base flows (Topping et al., 2003); flow releases through the power plant also contain seasonal and daily cycles that follow electricity demand in the western U.S. (White et al., 2005). Environmental flow constraints were imposed on the dam’s power plant operation after 1991 due to perceived effects on downstream physical, biological, and cultural resources (U.S. Department of the Interior, 1995, 1996). These “Record-of-Decision” (ROD) operations reduced the daily range and daily peaks of fluctuating flows, and subsequent experimental releases have included low steady flows and large simulated floods. However, more than four decades after construction of this large dam (the fourth highest in the United States) and after considerable scientific research and monitoring, scientists and river stakeholders continue to debate its impacts on downstream resources and how to reverse those environmental conditions that are deemed undesirable by society (Lovich and Melis, 2005; Melis et al., 2006).

One of the distinctive environmental attributes of the pre-dam river are sandbars that form in large eddies downstream of tributary debris fans (Schmidt, 1990; Schmidt and Rubin, 1995; Fig. 2) and floodplain-like channel-margin deposits.

\*E-mails: sawright@usgs.gov; jschmidt@cnr.usu.edu; tmelis@usgs.gov; dtopping@usgs.gov; drubin@usgs.gov

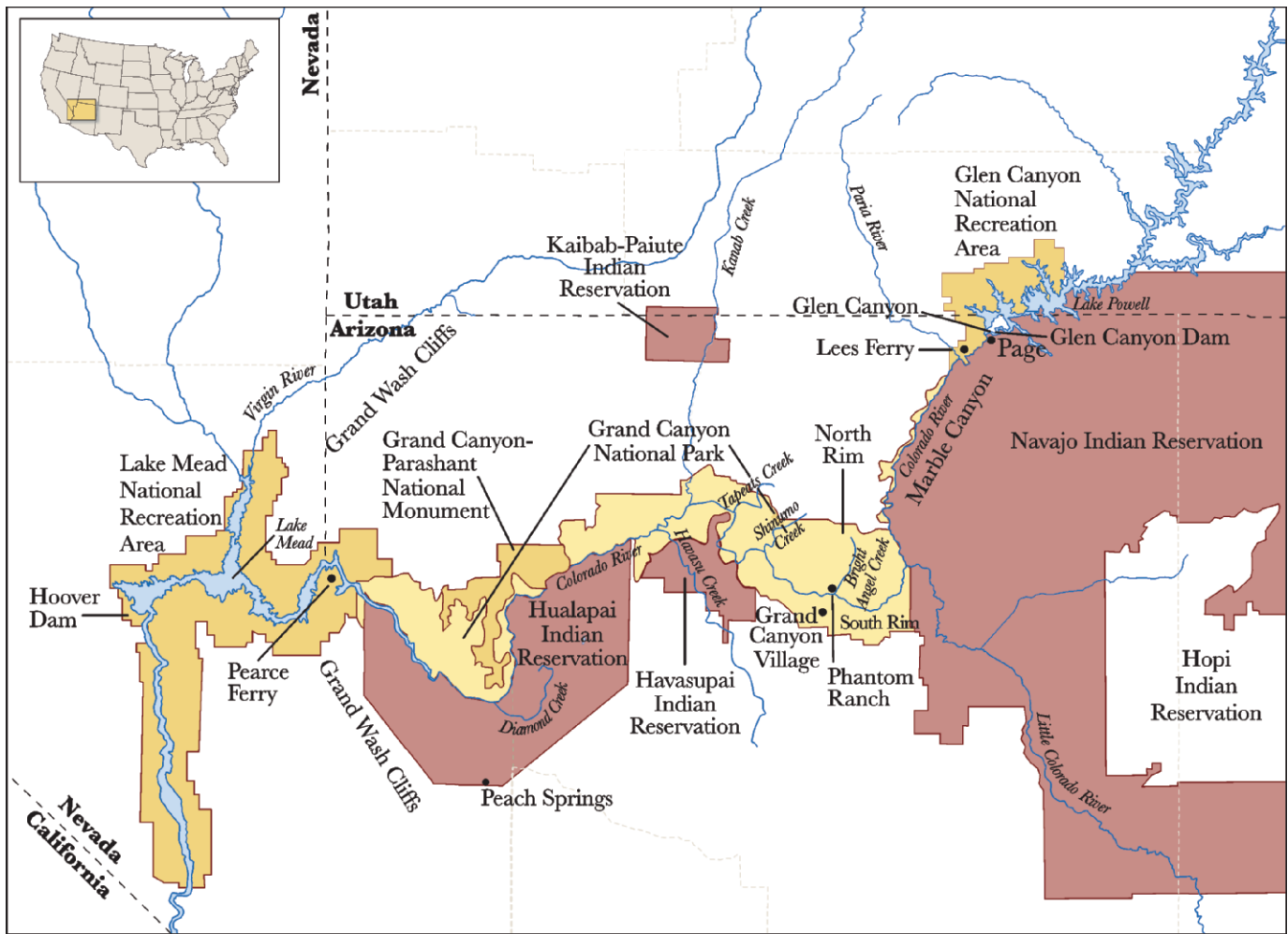


Figure 1. Location map of the Colorado River between Lakes Powell and Mead in northern Arizona, USA. Marble Canyon is the reach of river between Lees Ferry and the confluence with the Little Colorado River.

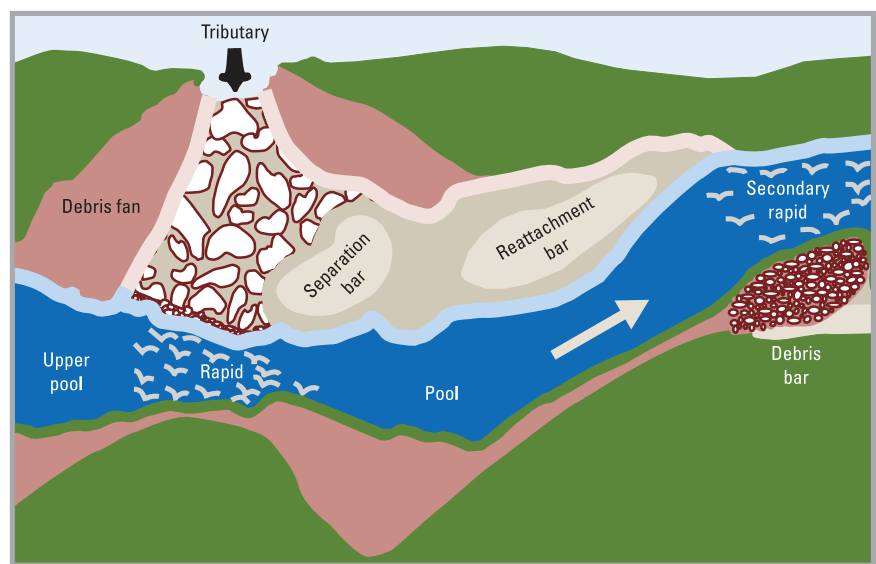


Figure 2. Example of a recirculating eddy and sandbar formed around the flow reattachment point along the Colorado River below Glen Canyon Dam. Image on right reproduced from Webb et al. (2005).



These sandbars are considered valuable resources by stakeholders of the Glen Canyon Dam Adaptive Management Program ([www.usbr.gov/uc/rm/amp/index.html](http://www.usbr.gov/uc/rm/amp/index.html)) for a variety of reasons: they are a fundamental element of the pre-dam riverscape within Grand Canyon National Park; they provide Park visitors, such as river runners and hikers, with recreational areas; they create zones of low-velocity aquatic habitat for juvenile native fish; they are the substrate for unique but limited riparian vegetation in an arid setting; and they are a source of sand for upslope wind-driven transport that may help protect archaeological resources.

Several attempts have been made at evaluating the post-dam sand budget; i.e., determining whether or not the amount of sand supplied by tributaries downstream of the dam exceeds the capacity of the regulated river to transport sand (see review in Wright et al., 2005). The numerous studies of the post-dam sand budget have come to conflicting results, but recent studies have concluded that the Colorado River between the dam and Phantom Ranch (Fig. 1) was evacuated of fine sediment from much of its bed and large parts of its eddies during the first three decades after dam closure and that erosion continues at a much slower rate today (Rubin et al., 2002; Schmidt et al., 2004; Wright et al., 2005; Webb et al., 2002). Because sandbars have continued to erode under ROD operations, the primary sediment-related question identified by scientists at a knowledge assessment workshop in July 2005 (Melis et al., 2006) was, “Is there a ‘flow-only’ operation that will rebuild and maintain sandbar habitats over decadal time scales?”

The goal of the analysis presented here is to evaluate what might be considered the “best-case flow-only scenario” for reversing the trend of sandbar erosion without sediment augmentation from sources in the upstream reservoir (Randle et al., 2007). Our analysis is conceptual but nevertheless reveals whether there is any reasonable expectation that a flow-only scenario can rebuild and maintain eroding sandbars in the post-dam era. If there is, then this analysis provides a basis for further analyses of other scenarios and sets a best-case expectation for the rate at which sandbars could be rebuilt. If not, then the analysis sets the stage for managers’ value-based discussions related to costs and benefits associated with non-flow options such as sediment augmentation. The best-case flow-only scenario is relatively easy to evaluate with existing knowledge as compared to more complex scenarios, and this analysis helps to identify areas of uncertainty that require future research.

## OPTIMAL DAM OPERATIONS FOR REBUILDING SANDBARS

For the analysis presented herein, flow releases from Glen Canyon Dam can be broken into two components: (1) short-duration (on a time scale of days) dam releases that are substantially higher than “normal” releases—herein referred to as “high flow events”; and (2) intervening, or “normal,” operations for the remainder of the year. The intervening operations control the amount of sand that is retained, if any, in the reach during tributary flooding and the amount that is exported during periods of tributary quiescence. High flow events are required to transfer sand from the channel bed and low-elevation parts of sandbars, where sand is first deposited when a tributary brings

sediment into the Colorado River, to higher-stage environments inundated only during floods, such as high-elevation parts of sandbars and other channel-margin deposits.

The “optimal” intervening dam operation is that which will result in the most tributary sand being available in the main-stem Colorado River for redistribution during high flow events. It is not difficult to specify this optimum operation because sediment transport theory dictates that a steady flow will transport less sand than an equivalent-volume fluctuating flow (e.g., ASCE, 1975). Thus, dam releases that vary seasonally and daily to meet electricity demand, such as approved by the ROD, are not optimal for retaining sand on the river bed prior to redistribution to higher elevations by high flow events. Higher sand transport rates during fluctuating flows as compared to steady flows were recognized in the 1995 Environmental Impact Statement (U.S. Department of the Interior, 1995). Thus, for a given annual volume release, the optimal dam operation for accumulating tributary sand inputs is a constant, steady flow over the entire year. The optimal annual release volume for accumulating tributary sand is the minimum volume stipulated by the various agreements that govern water delivery between the upper and lower Colorado River basins.

It is more difficult to specify the optimal frequency and hydrograph shape of short-duration high flow events, because these releases do not optimize overall sand storage. That is, high flow events have been called a “double-edged sword” by Rubin et al. (2002), because they necessarily export relatively large volumes of sand in order to transfer sand to high-elevation portions of sandbars. Thus, the optimal use of high flow events is such that they maximize storage of sand in “desirable” environments while attempting to minimize downstream export. Because we are currently unable to precisely define this optimal use, we assume an annual high flow event with a hydrograph shape that is dictated by the amount of sand that has accumulated in a given year. For example, in years with very low tributary inputs there might be no high flow event or one with a small peak of only a few hours’ duration. Or, in a year with tributary inputs that are substantially above average, the high flow event may have a higher peak and longer duration. These details are not necessary for the analysis in the following section. Also, the timing of the annual high flow event is not important for our analysis so long as there is abundant sand available in the river channel below the dam from recent tributary floods.

## EVALUATION OF THE OPTIMAL DAM OPERATIONS

### Marble Canyon Sand Budget

We focus our analysis on Marble Canyon, the 100-km reach of the Colorado River situated between the confluences of the Paria River, the first major tributary below the dam, and Little Colorado River (Fig. 1), because this is the first reach downstream of the dam with a significant post-dam sand supply. A simple annual average sand budget for this reach is

$$\Delta M = M_{in} - M_{io} - M_{bf} \quad (1)$$

where  $\Delta M$  is the change in sand mass in the reach,  $M_{in}$  is the incoming mass of sand from the Paria River and other smaller

tributaries,  $M_{io}$  is the mass of sand exported during intervening operations, and  $M_{hf}$  is the mass exported during high flows. The mass of sand coming into the reach from upstream (Glen Canyon) is assumed to be zero because releases from the dam contain almost no sediment, and the 25-km-long reach from the dam to Lees Ferry is substantially depleted of sand (Grams et al., 2007). Since the tributary inputs will almost always occur during intervening operations,  $M_{in}$  and  $M_{io}$  can be combined into a term representing the change in storage during intervening operations:

$$\Delta M = \Delta M_{io} - M_{hf} \quad (2)$$

where  $\Delta M_{io} = M_{in} - M_{io}$ . From (Eq. 2) it is apparent that in order for the overall storage in the reach to be positive, the export during high flow events must be less than the amount of sand that accumulates in the reach during intervening operations. It follows that if  $\Delta M_{io}$  is negative, then no sand would be available, on average, for redistribution during high flows, and it would thus be impossible to rebuild and maintain sandbars. The first step, then, in evaluating the optimal dam operation is to estimate the potential sand accumulation during intervening operations (i.e., the sign and magnitude of  $\Delta M_{io}$ ).

### Accumulation during Intervening Operations

The average annual sand supply from tributaries ( $M_{in}$ ) is relatively well constrained. The Paria River is the primary sand supply to Marble Canyon (Fig. 1) and delivers ~1,500,000 metric tons of sand per year (t/yr) (Topping et al., 2000a). Smaller tributaries are estimated to supply ~290,000 t/yr to Marble Canyon (Webb et al., 2000), resulting in total tributary sand supply of ~1,800,000 yr. Pre-dam, ~23,000,000 yr of sand was supplied from the watershed upstream from the Paria River (Topping et al., 2000a), such that the post-dam sand supply to Marble Canyon is ~7%–8% of the pre-dam supply.

In order to estimate export during intervening operations, the year-round steady flow rate must be specified. This rate depends on the minimum annual release volume from the dam. In December 2007, the U.S. Secretary of the Interior signed a ROD that implemented interim guidelines for the coordinated operation of Lakes Powell and Mead (U.S. Department of the Interior, 2007a). These interim guidelines stipulate a minimum objective release volume of  $10.2 \times 10^9$  m<sup>3</sup>/yr (8.23 million acre-feet [MAF]) as well as operational tiers under which the annual release may be reduced to as low as  $8.6 \times 10^9$  m<sup>3</sup> (7.0 MAF). However, the final environmental impact statement concludes that during the period 2008–2026, the probability of releases being below the minimum objective is ~10% (U.S. Department of the Interior, 2007b, figures 4.3-13, p. 4-43). Because of this relatively low probability and the uncertainty surrounding future hydrologic conditions in the basin (Barnett et al., 2008; Seager et al., 2007), we chose to evaluate the minimum objective release volume of  $10.2 \times 10^9$  m<sup>3</sup> (8.23 MAF), which, averaged over the entire year, equates to a steady flow of 322 m<sup>3</sup>/s. We also note that this has been the annual release volume since 2001 due to drought conditions and low reservoir levels.

The next step is to estimate the sand concentration at the steady 322 m<sup>3</sup>/s water discharge. Several previous studies have attempted

to define a relationship between water discharge and sand concentration, such as Randle and Pemberton (1987), upon which the ROD operation was based. Subsequent research, however, has shown that the relationship between suspended-sand concentration and water discharge is not constant in this reach of river; rather, substantial shifts occur due to changes in the upstream supply of sand (Topping et al., 1999, 2000a, 2000b, 2005, 2007; Rubin and Topping, 2001). These shifts make the estimation of sand concentration at a steady 322 m<sup>3</sup>/s water discharge a more complicated affair than in rivers where discharge and sediment supply are more in equilibrium. That is, some assumptions regarding the state of sand supply must be made in order to estimate the sand concentration.

Figure 3 shows suspended-sand concentration versus water discharge as measured at two gages in Marble Canyon between August 1999 and January 2008, illustrating the wide range in sand concentration that can occur for a given water discharge. The 30-mile gage is located in middle Marble Canyon, and the 61-mile gage is located at the lower end of Marble Canyon (see Topping et al., 2006a, 2006b). At the 30-mile gage, sand concentrations for flows within  $\pm 5\%$  of 322 m<sup>3</sup>/s ranged from 9 to 104 mg/L, with median, mean, and standard deviation of 51, 51, and 31 mg/L, respectively. At the 61-mile gage, sand concentrations ranged from 11 to 150 mg/L, with median, mean, and standard deviation of 23, 34, and 29 mg/L, respectively. Figure 4 illustrates the effects of sand supply on sand concentration by plotting the concentrations contained in the box of Figure 3 along with the daily water discharge record for the Paria River (U.S. Geological Survey gage 09382000). During periods of little tributary activity, when the sand budget tends to be negative under ROD releases, such as 2001–2003 (Wright et al., 2005), the sand concentrations are lower. During periods

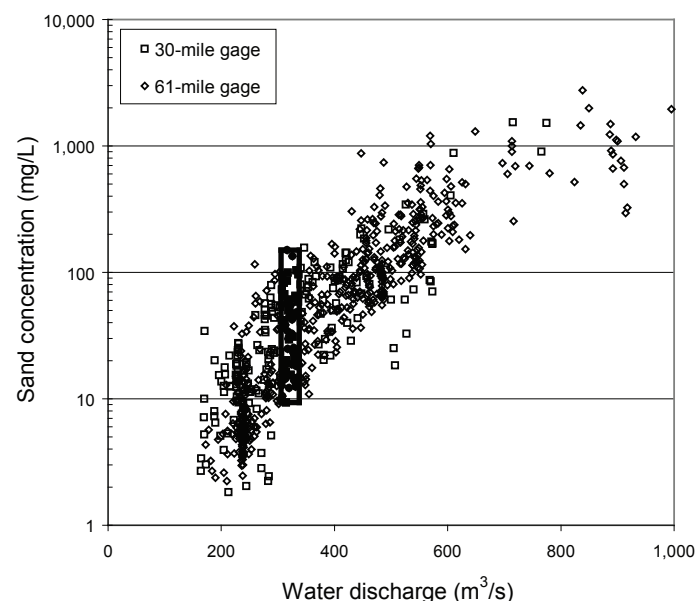


Figure 3. Suspended-sand concentration versus water discharge as measured in middle Marble Canyon (30-mile gage) and at the lower end of Marble Canyon (61-mile gage) for August 1999–January 2008. Bold rectangle encloses measurements made in the water discharge range of 322 m<sup>3</sup>/s  $\pm 5\%$ .

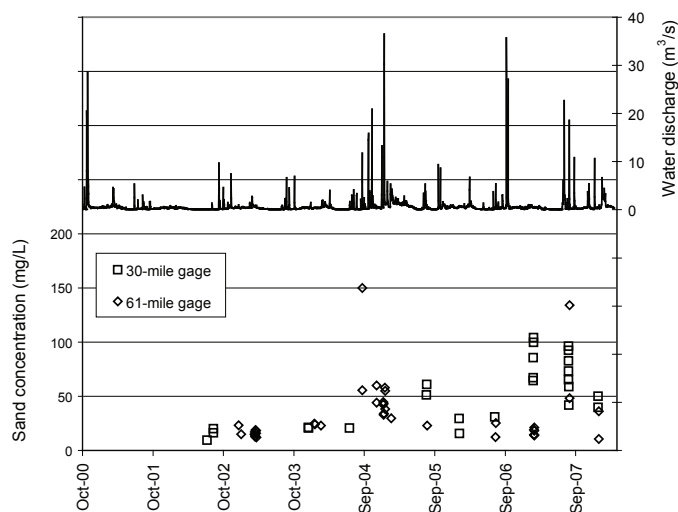


Figure 4. Paria River water discharge (top) and mainstem Colorado River sand concentrations for mainstem discharges in the  $322 \text{ m}^3/\text{s} \pm 5\%$  range (bottom) for October 2000–April 2008. Sand concentrations increased during temporary sand accumulation in Marble Canyon following Paria floods in fall 2004, winter 2005, and fall 2006 and subsequently decreased as the tributary sand was exported from Marble Canyon.

of substantial tributary flooding and temporary accumulation of sand in Marble Canyon, such as fall 2004 (Topping et al., 2006b), winter 2005, and fall 2006 (U.S. Department of the Interior, 2008), concentrations are higher because sand supply is greater.

Because the steady  $322 \text{ m}^3/\text{s}$  operation is expected to transport substantially less sand than ROD operations, the sand supply should be greater on average such that sand concentrations under this operation would tend toward the high end of those measured during 1999–2008. Given this, our best estimate is based on the period from October 2006–January 2008, because tributary inputs were above average, dam releases were minimum annual volumes, and substantial accumulation of sand occurred in Marble Canyon (U.S. Department of the Interior, 2008). The median sand concentration measured during this period, based on both Marble Canyon gages, was  $54 \text{ mg/L}$  (Fig. 4), which when combined with steady  $322 \text{ m}^3/\text{s}$  yields an annual sand flux of  $\sim 500,000 \text{ t}$ . This is the value we have used for  $M_{io}$  in subsequent calculations while acknowledging substantial uncertainty in the estimate due to the difficulty in projecting future sand supply conditions.

### Redistribution during High Flow Events

Using these estimates for tributary inputs ( $1,800,000 \text{ t}$ ) and export during intervening operations ( $500,000 \text{ t}$ ), the potential accumulation in Marble Canyon,  $\Delta M_{ior}$ , is  $\sim 1,300,000 \text{ t}$ . This is the mass of sand, on average, that would be available for redistribution during a high flow and is near the upper bound of the estimate for sand storage in upper Marble Canyon going into the November 2004 high flow (Topping et al., 2006a). In the pre-dam river, Topping et al. (2000a) estimated seasonal fine sediment accumulation in an average year of  $\sim 7,000,000 \text{ t}$ , such that the potential accumulation during intervening dam operations described herein is  $\sim 20\%$  of the estimated pre-dam sea-

sonal accumulation. There would be substantial variation in the amount of sand accumulation from year to year owing to variability in tributary inputs, but we have chosen to focus on the average response instead of the variability because it provides more insight with respect to the long-term prospects for rebuilding and maintaining sandbars.

The degree to which sandbars can be rebuilt depends on how much of the sand that accumulates during intervening operations remains in sandbars following a high flow event. That is, what fraction of the accumulated sand is exported downstream, eventually to Lake Mead, versus deposited in sandbars? Topping et al. (2006b) estimated that  $\sim 10\%$ – $20\%$  of the tributary sand that had accumulated in upper Marble Canyon was still in the reach following the 2004 high flow, presumably in sandbars. Though the 1996 high flow event resulted in an overall net loss of sediment from sandbars (Schmidt, 1999; Hazel et al., 2006), there was a gain in high-elevation volume  $\sim 20\%$  of that was the losses from the low-elevation portions of sandbars and the channel. Hazel et al. (2006) estimated the potential active storage in sandbars in Marble Canyon to be  $\sim 13,000,000 \text{ t}$ , or  $\sim 20\%$  of the total pre-dam fine sediment load. Thus, it appears that a reasonable estimate for the fraction of sand transferred to sandbars during high flow events is  $\sim 10\%$ – $20\%$  of the available supply. It follows, then, that  $\sim 200,000 \text{ t/yr}$  of the  $1,300,000 \text{ t/yr}$  of potential sand accumulation could go toward building sandbars.

### Comparison with Post-Dam Erosion Rates

Several existing data sets and previous analyses facilitate comparison of the potential annual accumulation in sandbars with the rate of sandbar erosion since dam closure. Using daily sediment records from gages near Lees Ferry and Phantom Ranch (Fig. 1), Rubin and Topping (2001) estimated that  $16,000,000 \text{ t}$  of fine sediment was eroded from this reach during high flow releases in April–June 1965. It is impossible to know precisely how much of this sediment came from sandbars versus the channel bed, but even a conservative estimate of  $10\%$  from sandbars yields  $1,600,000 \text{ t}$  of erosion during this three-month period, which is eight times our estimated potential annual accumulation. Schmidt et al. (2004) and Hazel et al. (2006) estimated the loss of fine sediment from sandbars from the pre-dam era through the 1990s to be  $\sim 6,000,000 \text{ t}$ . Over the  $\sim 40$  years since dam construction, this equates to an average annual erosion rate of  $\sim 150,000 \text{ t}$ , which is in the same range as our estimated potential annual accumulation in sandbars ( $200,000 \text{ t}$ ). Thus, our analysis indicates that under the “best-case scenario” for hydrologic conditions and dam operations, sandbars could potentially be rebuilt, at least initially, at approximately the same rate as they have eroded since dam construction.

### DISCUSSION

Though our analysis suggests potential annual sandbar accumulation at about the same rate as the post-dam erosion rate, we do not mean to imply that pre-dam conditions could be achieved in 40 years; this is almost certainly not the case. The cumulative rate of sandbar building depends on how much of the annual accumulation is maintained between high flow events, how the system changes as accumulation occurs over



time, and the frequency and magnitude of deviations in future annual release volumes from the minimum objective volume. Our analysis evaluates only the potential for accumulation in a given “average” year and does not address cumulative accumulation because there is too much uncertainty about how the system would evolve over multiple years, or even decades, under these operations. For example, we know that as accumulation occurs, sand export during intervening operations will increase, but we do not know by how much. Also, we have very little information on how stable the sandbars would be under steady flows. Finally, the probability that annual releases will exceed the minimum objective release in a given year is ~60% (U.S. Department of the Interior, 2007b, figure 4.3-13, p. 4–43) such that incremental gains in sandbar building made during periods of minimum objective releases would be subjected to higher flows more than half the time in the future.

The question remains open as to the viability of operations that deviate from the optimal conditions that we have defined, such as the current ROD operations that contain seasonal and daily variations in flow releases. One of the advantages of the operation we have defined is that it is simple enough that it can be evaluated relatively quickly with our existing knowledge. More complicated scenarios that include variability in upper basin hydrology, dam releases, and tributary sand supply become much more difficult to evaluate, particularly over extended periods, and require models that are as yet not available. Currently, all that we can say is that deviations from the optimal operation, such as annual volumes that exceed minimum objective releases and/or seasonal and daily flow variations, will either reduce the rate of sandbar building or lead to continued erosion instead of accumulation. That is, the sandbar rebuilding rate under the optimal operation can be considered an upper bound.

The ability to evaluate more complex operational scenarios will require future research, and one of the reasons for conducting the analysis presented here was to identify areas with the greatest uncertainty. Considerable research and monitoring has been conducted to define the magnitude of tributary inputs and the rate at which these inputs are exported from the system by ROD dam releases such that we feel these components of the analysis are relatively well constrained. The greatest uncertainty lies with our understanding of the processes of sandbar building during high flows and readjustment during subsequent flows. For example, our estimate for the transfer of sand from the channel to sandbars during high flows is made on the basis of only two experimental releases, conducted in 1996 and 2004 (data from a third experimental high flow test in March 2008 are still being collected and analyzed as of this writing). Also, significant uncertainty exists as to the rate at which recently deposited sandbars are eroded by subsequent flows, particularly under steady flows.

## CONCLUSIONS

Despite extensive research and monitoring over the past several decades, the question of whether sandbars can be rebuilt and maintained in a sustainable manner along the Colorado River in Grand Canyon below Glen Canyon Dam has remained unanswered. Here, we have drawn from this extensive litera-

ture to conduct a relatively simple analysis of the potential for rebuilding sandbars under what might be considered a “best-case scenario” in terms of hydrologic conditions and dam operations. Our analysis suggests that this scenario may indeed have viability for rebuilding sandbars. We estimate a potential sandbar rebuilding rate of ~200,000 t/yr, which is comparable to the estimated average sandbar erosion rate since dam construction of ~150,000 t/yr. However, the similarity in these rates should not be used to infer that pre-dam conditions could be achieved in 40 years. Potential cumulative sandbar building would depend on how much of the annual accumulation is maintained between high flow events, how the system changes through time as accumulation occurs, and the frequency and magnitude of deviations in future annual release volumes from the minimum objective release. Deviations from our best-case scenario, such as seasonal variability in flow releases, daily flow fluctuations, or greater than minimum release volumes would either reduce the sandbar accumulation rate or result in continued erosion of sandbars.

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Manuscript received 11 February 2008; accepted 28 April 2008. ☞

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# Corresponding PowerPoint Presentation

# Is there enough sand?

## Evaluating the fate of Grand Canyon sandbars

Scott A. Wright, John C. Schmidt, Theodore S. Melis,  
David J. Topping, David M. Rubin

9 Sep 2008

GCDAMP AMWG meeting, Flagstaff



# Motivation

- ❖ Science Symposium Panel (Oct 2005): Several were declaring it a lost cause and that sediment augmentation is needed
- ❖ Knowledge Assessment Workshop (Aug 2005):  
Primary sediment question: “Is there a ‘flow-only’ (non sediment augmentation) operation that will rebuild and maintain sandbar habitats over decadal time scales?”

**Goal was to take a “big picture”  
approach to try to answer the question,  
Is there enough sand?**



# Approach

- ❖ Define and evaluate the “optimal conditions”, in terms of hydrology and dam operations for rebuilding and maintaining sandbars
- ❖ Why this approach?
  - ❖ It's the logical first step - If the "optimal operations" don't “work”, then non-flow measures may be evaluated. If the "optimal operations" do “work”, it sets the upper bound for expectations and leads into more complex scenarios
  - ❖ The "optimal operations" are the easiest to analyze with available information, leading to the least uncertainty

# Definition of "optimal operations"

Sand transport is non-linearly related to water discharge. It follows that lower flows transport less sand, and for a given flow volume, steady flows transport less sand than fluctuating flows.

Thus, for maximizing sand retention:

- “optimal hydrology” is minimum volume releases (8.23 MAF\*)
- “optimal flows” are steady year-round
- result: 8.23 MAF ~ steady 11,400 cfs

Any accumulation would be below 11,400 cfs stage. Redistribution to higher elevations is required by periodic high flows, e.g. every 1-3 years depending on tributary inputs.



\* This analysis was done before coordinated operations criteria

# Approach

Annual sand budget for Marble Canyon

$$\Delta M_{io} = M_{in} - M_{io}$$

$M_{in}$  is tributary inputs,  $M_{io}$  is export during “intervening operations”, i.e. between high flows

In order to have sustainable sandbar building, annual accumulation during intervening operations is required, i.e.  $\Delta M_{io} > 0$

First step, then, is to evaluate potential accumulation during intervening operations,  $\Delta M_{io}$

# Annual tributary inputs

$$\Delta M_{io} = M_{in} - M_{io}$$

$M_{in}$  – annual tributary inputs

Paria ~ 1,500,000 metric tons per year (Topping et al., 2000)

Other tribs ~ 290,000\* metric tons per year (Webb et al., 2000)

Total ~ 1,800,000 metric tons per year



\* recent data indicate this may be as much as a factor of two high

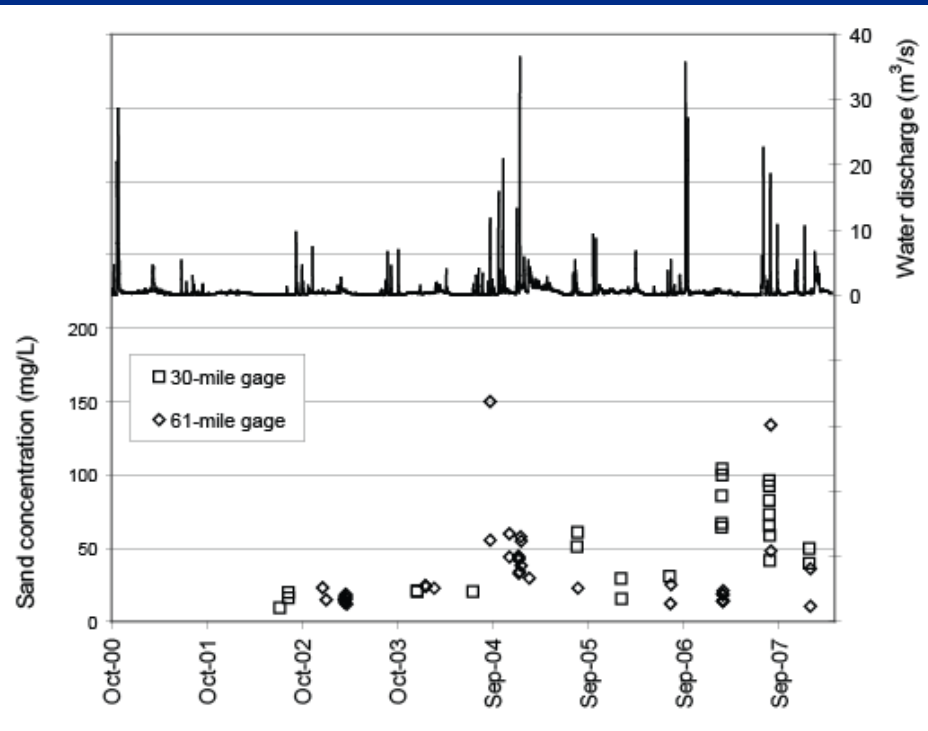


# Annual export (non high flow)

$$\Delta M_{io} = M_{in} - M_{io}$$

$M_{io}$  – annual export (non high flow)

To compute  $M_{io}$ , we need to estimate the sand concentration at 11,400 cfs



Median measured sand concentrations for  $\sim 11,400$  cfs flows in Marble Canyon from Oct 06 – Jan 08 = 54  $\text{mg/L}$

This results in:  
 $M_{io} \sim 500,000$  metric tons per year

# Annual accumulation

$$\Delta M_{io} = M_{in} - M_{io}$$

$$\Delta M_{io} = 1,800,000 - 500,000 = \underline{1,300,000 \text{ metric tons per year}}$$

Thus, “optimal operations” have the potential to accumulate about 1.3 million metric tons per year (on average)

This sand would be below the 11,400 cfs stage; requires redistribution to higher elevations by high flows

Next question: How much can be retained in sandbars?

# High flow redistribution

As pointed out by Rubin et al. (2002), high flows are a “double edged sword” because, in order to redistribute sand to higher elevation, a substantial quantity must be exported:

$$(F_b + F_c + F_e)\Delta M_{io}$$

Fraction  
transferred  
to bars

Fraction left  
behind in  
channel

Fraction  
exported

# High flow redistribution

- ❑ Topping et al. (2006b) estimated that ~10-20% of the available tributary sand retained in the upper 50 km of Marble Canyon was still in the reach following the 2004 high flow event, presumably in sandbars
- ❑ Though the 1996 high flow event resulted in an overall net loss of sediment from sandbars (Schmidt, 1999, Hazel et al., 2006), there was a gain in high-elevation volume that was ~20% of the losses from the low-elevation portions of sandbars and the channel
- ❑ Hazel et al. (2006) estimated the potential active storage in sandbars in Marble Canyon to be ~13,000,000 metric tons, which is ~20% of the total pre-dam fine sediment load (sand and finer)

Based on these estimates, we chose  $F_b \sim 0.15$

# High flow redistribution

$$\text{Sandbar accumulation} = F_b \Delta M_{io}$$

$$F_b = 0.15 \text{ and } \Delta M_{io} = 1,300,000 \text{ metric tons}$$

Thus, potential sandbar accumulation ~ 200,000 metric tons per year. Since this is based on “optimal” operations, it can be considered the upper bound for expectations

If accumulation is maintained until the next high flow, response could be cumulative through time

**Context: How does this compare to post-dam erosion rates?**



# Post-dam erosion rates

- ❑ Schmidt et al. (2004) and Hazel et al. (2006) estimated the loss of fine sediment from sandbars from the pre-dam era through the 1990s to be ~6,000,000 metric tons.
- ❑ Over the approximately 40 years since dam construction, this equates to an average annual erosion rate of ~150,000 metric tons per year, which is comparable to the potential sandbar accumulation rate (~200,000 metric tons per year).
- ❑ Erosion rates have not been constant over time. For example, high flows in April-May 1965 resulted in ~16,000,000 metric tons of erosion from Marble Canyon (Rubin and Topping, 2001). If 10% came from sandbars, this equates to 25% of the total erosion.

# Conclusions

- ❖ The “optimal conditions” for hydrology and dam operations has some viability for rebuilding and maintaining sandbar deposits
- ❖ The near-term rate at which sandbars could potentially be rebuilt is comparable to their erosion rate over the ~40 years since dam construction. This does not mean pre-dam conditions could be achieved in 40 years; the rate of accumulation would decrease as sandbars are rebuilt
- ❖ Deviations from “optimal conditions” (e.g. wetter hydrology, fluctuating flows) will decrease the rate of accumulation or result in net erosion
- ❖ The largest uncertainty is in estimating  $F_b$ . Can high flow hydrographs be “optimized” to promote sandbar building while minimizing export?

# Future work

- ❖ Variable hydrology – We evaluated only 8.23 MAF release volumes, but future releases will vary dependent on basin hydrology. This presents great difficulty for evaluating the long-term sand budget since future hydrology is unknown and hard to predict.
- ❖ Fluctuating flows – To evaluate a year-round steady flow, one discharge and one concentration are required. To evaluate fluctuating flows, detailed hydrographs and more complex models are required. We are making progress on this end (November science symposium).
- ❖ Optimized high flow hydrographs and sediment triggers – Need models for evaluating various hydrograph scenarios and supply conditions. Data collected in March are being used to develop and test these models.

# Back to the original question:

Is there a 'flow-only' (non sediment augmentation) operation that will rebuild and maintain sandbar habitats over decadal time scales?

We think so, yes...

BUT: It is likely dependent on low volume releases and/or steady flows

AND: It will take time even under "optimal operations"

AND: There is an upper limit to the degree of bar building that can be achieved