UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF RECLAMATION

BUREAU OF RECLAMATION HYDRAULIC LABORATORY

HYD 553

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VERTICAL ADJUSTABLE WEIR

Report No. Hyd-553

Hydraulics Branch DIVISION OF RESEARCH



OFFICE OF CHIEF ENGINEER DENVER, COLORADO

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ABSTRACT

Discharge measurement accuracy of a vertically adjustable Cipolletti weir having a 2-ft crest length was investigated in the laboratory. The investigation included determination of head-discharge relationship of the weir and a means of standardizing the method of relating elevation of the movable weir crest to head of water on the crest. Discharges indicated by the weir head were in general in good agreement with discharges measured by volumetrically calibrated Venturi meters for movable crest elevations greater than 0.1 ft above the stationary weir. Reliable discharge measurements could not be made for movable weir crest elevations less than about 0.1 ft above the stationary crest.

DESCRIPTORS-- irrigation canals/*Cipolletti weirs/*laboratory tests/
*discharge measurement/water measurement/staff gages/weir crests/
discharge coefficients/turnouts/check structures/aeration/nappe/
leakage/research and development/Venturi meters
IDENTIFIERS-- adjustable weirs

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VERTICAL ADJUSTABLE WEIR

SUMMARY AND CONCLUSIONS

A 2-foot adjustable Cipolletti weir was installed in a 4-foot wide flume in the laboratory, Figures 1 and 2. The weir was installed according to the recommendations listed in the first edition of the Water Measurement Manual published by the Bureau of Reclamation.

The purpose of the investigation of the weir was to study the flow characteristics and establish a head-capacity curve for the normal discharge range of a 2-foot Cipolletti weir. A means of standardizing the method of relating the adjustable crest elevation to the head above the fixed crest was developed as part of the study.

Discharges measured with the adjustable Cipolletti weir were in good agreement with discharges measured by Venturi meters volumetrically calibrated in the laboratory. For adjustable crest elevations 0.1 foot and greater above the fixed crest, the discharge indicated by the weir exceeded that measured by the Venturi meters by about 6 percent at a head of 0.2 foot; and 1 percent at a head of 0.6 foot, Figure 6. For weir crest elevations less than about 0.1 foot, the indicated discharges of the weir were less than measured by the meters and the difference in discharges ranged to about 15 percent, Figure 7 and 8. Leakage between the adjustable crest plate and the fixed crest plate was appreciable and helped to reduce the weir accuracy, Figure 5.

The use of two enameled staff gages is proposed for standardizing the method of measuring the weir head. One gage is mounted in the weir pool in a position which is unaffected by the velocity of approach, and the second is attached to the frame which is part of the movable weir blade. Both gages are referenced to the elevation of the fixed crest, Figure 2B, so that a pointer attached to the movable plate would give a direct reading of the crest elevation, and the difference in readings between the upstream and frame-mounted gages, using the water surface as an indicator would give the weir head. This system of gages and pointer proved to be the most satisfactory means of measuring the head above the weir crest.

In irrigation systems where the necessary head is available, the adjustable Cipolletti weir can be used for regulating and measuring discharges with a reasonable degree of accuracy.

When more than one weir is contained in a division box, Figure 4, the necessary proximity of the weirs could have a detrimental influence on the head-capacity relationships of each weir, depending on the weir discharges and the size of the division box. This influence would undoubtedly vary as the separate weirs are adjusted through their ranges of discharge. Whether this influence would be greater than the acceptable range of discharge measurement accuracy could probably be determined by laboratory tests.

INTRODUCTION

Two types of adjustable weirs have been employed for the purpose of measuring and regulating flow in irrigation systems. The first has a horizontal fixed crest and adjustable vertical sides which may be moved to change the size of the rectangular opening. The sides are adjusted by hand, making fine adjustments quite cumbersome.

The Bureau of Reclamation uses a second type called a vertical adjustable weir, Figures 1, 2, 3, and 4. A study of the available literature indicates the weir to be unique in its application to irrigation. The weir assembly includes a movable plate with a Cipolletti weir notch cut out of the center portion (movable weir); another fixed plate (stationary weir) has a large rectangular notch cut out of the central area, Figure 1. The movable weir plate bears against the stationary weir plate and can be adjusted vertically by means of an Acme thread screw and a handwheel placed on a frame at the top of the structure. Although the weir serves as a measuring device, the primary purpose is to control or "check" the flow in a lateral or pump turnout where a variable depth is probable, Figures 3 and 4.

Standardization of the construction details has been accomplished for 2-, 3-, and 4-foot adjustable Cipolletti weirs using an economical design. No reports on malfunction have been found; however, leakage between the fixed and movable plates has been a minor problem in some cases. A complicating factor in the Columbia Basin system is that laterals usually terminate in a rectangular weir box, 5 feet square. The necessary proximity of the two or three adjustable weirs in this box might influence the relative discharge from the weirs and the degree of influence would undoubtedly vary as the separate weirs are adjusted through their range of openings. Whether the discharge error would be greater than the acceptable range of accuracy for the weirs could probably only be determined by laboratory test. However, this aspect of the overall problem was not included in the study.

INVESTIGATION

Laboratory Installation

A 2-foot adjustable weir, obtained on loan from the Black Canyon Irrigation District, Notus, Idaho, was installed in a 4-foot-wide flume in the laboratory, Figures 1 and 2. The flume provided weir pool dimensions equal to those recommended in the first edition of the Water Measurement Manual published by the Bureau of Reclamation. For a field installation, the thickness of the downstream support walls for the weir showed the possibility of affecting the flow by preventing aeration of the weir nappe. The concrete weir wall thickness shown in Section A-A of Figure 1 was simulated in the laboratory by a horizontal piece of wood on the downstream side and at the bottom of the opening of the fixed weir, Figure 2A.

In the original fabrication of the weir, a distortion of the adjustable crest blade prevented contact between the fixed and movable metal plates. In operation, water flowed between the plates. This water was not measured as part of the weir discharge. To correct this, the yoke and stem of the weir were removed to check the bolthole alinement. Inspection showed both a misalinement and a bent yoke. When the yoke was straightened, and the boltholes were alined, the clearance between the metal plate of the weir and the fixed plate was commensurate with the ordinary waviness of No. 12-gage galvanized iron. Leakage between the fixed and movable plates occurred for all positions of the weir crest, however, Figure 5.

Flow through the flume and weir was measured by volumetrically calibrated Venturi meters. The head corresponding to a measured flow through the movable weir was read from an enameled staff gage mounted 5 feet upstream on the side of the flume. A second enameled gage was firmly attached to the frame of the movable weir to provide an accurate indication of the elevation of the movable weir crest above the elevation of the crest of the fixed plate. A pointer attached to the movable plate allowed a direct reading of the crest elevation, Figure 2B. The difference in readings (using the water surface as an indicator) between the upstream gage and the one mounted on the weir frame, both having a zero reference to the elevation of the crest of the fixed blade, provided a measurement of the weir head. The discharge indicated by the weir head was then obtained from standard Cipolletti tables for a 2-foot weir for comparison to the discharge measured by the Venturi meter.

The system for measuring the movable weir crest elevation in the laboratory, as described in the preceding paragraph, differed from that which has been employed in the field. In field installations the adjustable weir crest is set at the same elevation as the fixed weir crest. A nut is turned down on the Acme thread screw to the top of the nut of the handwheel, Figure 1, and is then pinned or welded in place. After the weir blade is raised, the height of the adjustable crest above the fixed crest can be measured as the distance from the bottom of the nut to the top of the handwheel. The upstream staff gage, referenced at zero to the same elevation as the stationary weir crest, is used to measure the head on the fixed weir. The head on the movable crest is obtained by subtracting the stem measurement from the height of the water surface on the upstream staff gage. Discharges are then determined, using the head, from a Cipolletti weir discharge table. Both the laboratory and field methods of determining the weir head are satisfactory, but the one used in the laboratory can provide the greater accuracy of measurement.

Test Procedure

In a typical calibration test, the movable weir crest was set at the elevation of the top of the stationary crest and a constant flow was allowed to stabilize through the flume. The weir head was then measured on the staff gage located 5 feet upstream from the weir. The weir crest was then raised to elevations of 0.05, 0.1, 0.2, 0.3, 0.4, 0.5, and 0.6 foot above the stationary crest without adjusting the discharge, and the head on the weir was measured for each crest position. This procedure was repeated for eight flows, ranging from 0.06 cfs (cubic feet per second) to a flow of 3.42 cfs, corresponding to a maximum head of approximately one-third of the weir crest length. This is the maximum head recommended for accurate weir measurements.

The leakage flow between the weir bulkhead plate and the movable crest plate was not separately measured because such a measurement would apply directly only to the weir calibrated in the laboratory. It would not apply to other weirs having larger or smaller clearances. Thus, the discharge obtained from the tables for the measured head above the weir crest was not the total flow passing the weir structure but represented the difference between the Venturi measured flow and the leakage flow.

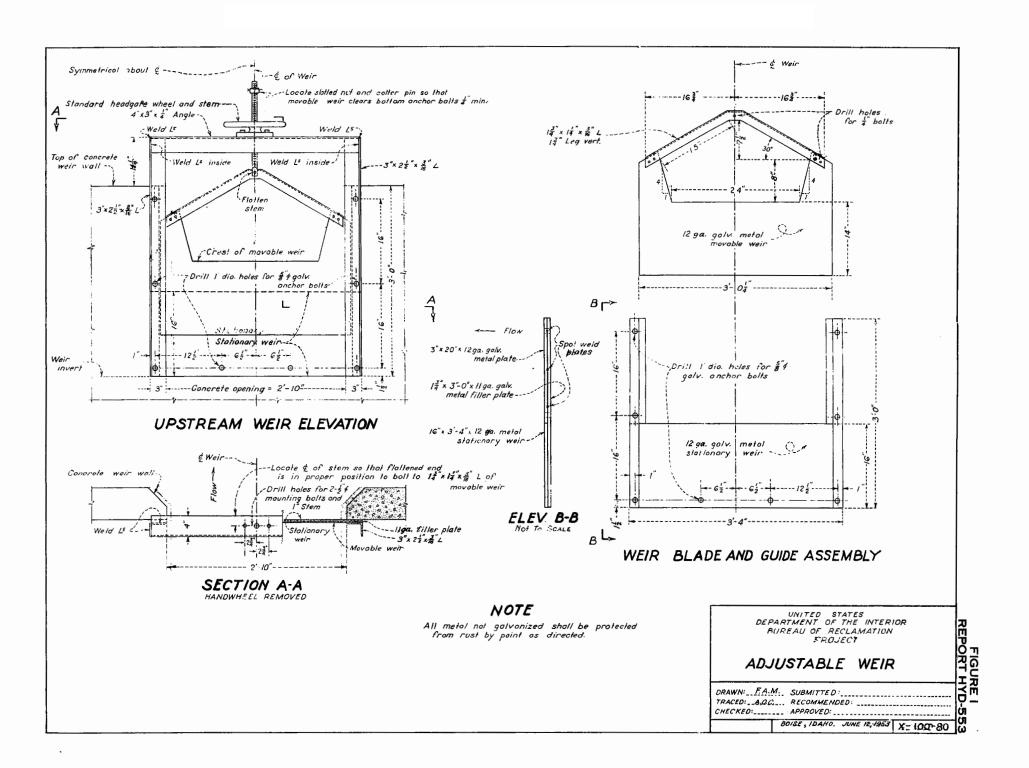
Test Results

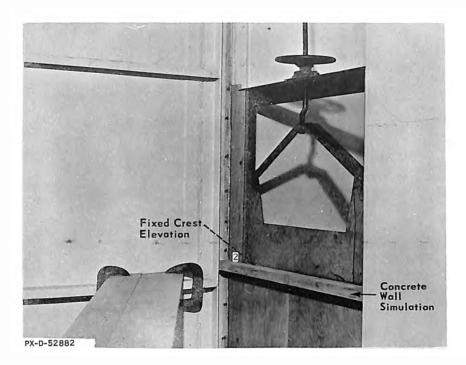
The data comparing the discharge measured by the Venturi meter and that indicated by the measured weir head were generalized in three graphs. Figure 6 is for the weir crest 0.10 foot or higher above the stationary crest; Figure 7 is for the weir crest 0.05 foot above the stationary crest; and Figure 8 is for the movable crest elevation equal to the stationary crest elevation.

Proper ventilation of the weir nappe occurred when the weir crest elevation was equal to or greater than 0.1 foot above the elevation of the fixed crest, Figure 5C to F. The discharge indicated by the weir was in good agreement with the Venturi measured discharge in the rated range of head (0.2 to 0.67 foot) for a 2-foot Cipolletti weir, although the total flow included the leakage, Figure 6. Use of the weir at crest elevations higher than 0.1 foot above the fixed crest would produce a means of controlling the flow and discharge measurement. The measurements in these tests showed the weir-indicated-discharge to be about 6 percent greater than the Venturi-measured discharge for a head of 0.20 foot and to be about 1 percent greater for a head of 0.6 foot.

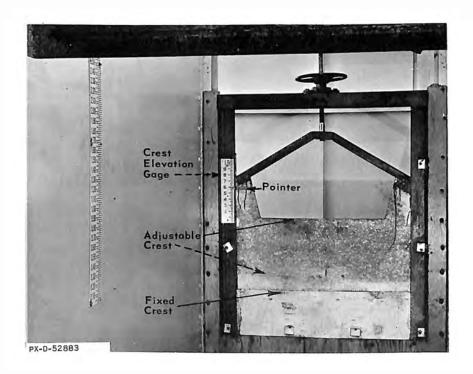
The agreement between the weir-indicated-discharge and Venturi meter was less satisfactory for weir crest elevations of 0.05 and 0.0 foot above the fixed crest elevation, Figures 7 and 8. For a crest elevation of 0.05 foot and heads larger than about 0.4 foot, the agreement between the discharges indicated by the weir and the discharges measured by the Venturi meter was good, Figure 7. Below a head of 0.4 foot, the differences between the measured discharges increased and the weir indicated about 10 percent less flow than the Venturi meter. The suspected cause was the interference between the lower surface of the nappe flowing over the weir crest and the upward flow of the leakage from between the weir plates, Figure 5B.

The discharges indicated by the weir ranged from about 15 percent to 0.5 percent greater than the Venturi for flows of 1 to 3 cfs, respectively, when the elevation of the weir and fixed crests were equal, Figure 8. For discharges below about 1 cfs, the difference between the weir and Venturi discharges was decreased and the weir indicated was about 10 percent less than the meter at a discharge of 0.6 cfs. The weir operating with the crest elevation equal to the fixed crest did not produce accurate discharge measurements, because the thickness of the walls supporting the adjustable weir assembly prevented proper aeration of the weir nappe, Figure 5A.





A. Downstream side of 2-foot adjustable weir



B. Staff gages on upstream side of adjustable weir

ADJUSTABLE CIPOLLETTI WEIR STUDY

Laboratory Installation of Weir



A. Four-foot adjustable weir used as a combination check and measuring structure--North Side Pumping Division Minidoka Project, Idaho

ADJUSTABLE CIPOLLETTI WEIR STUDY

Field Installation of Weir in Open Channel



A. Pumping plant and adjustable weirs installed in division box



B. Adjustable weirs in division box at discharge end of small pumping plant

ADJUSTABLE CIPOLLETTI WEIR STUDY

Weirs Installed in Pumping Plant Division Box Columbia Basin Project

Figure 5 REPORT HYD-553







A. 0.00 foot

B. 0.05 foot

C. 0.10 foot

Weir Crest Elevation above Fixed Crest







D. 0.20 foot

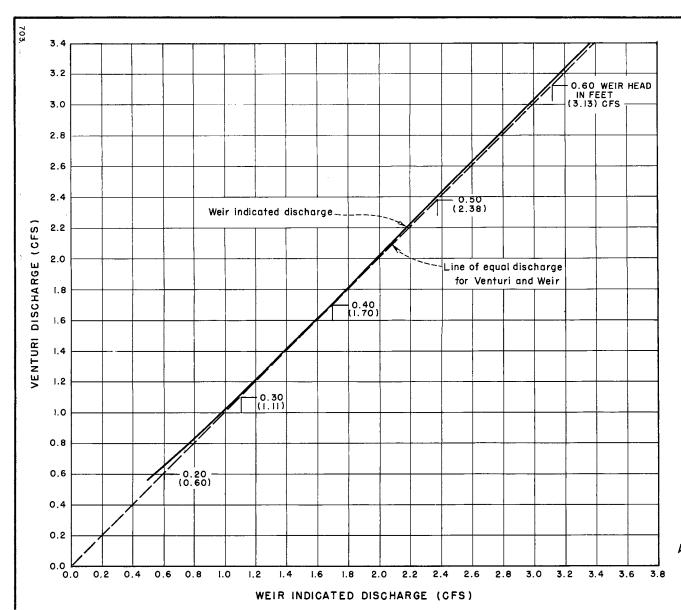
E. 0.40 foot

F. 0.60 foot

Weir Crest Elevation above Fixed Crest

ADJUSTABLE CIPOLLETTI WEIR STUDY

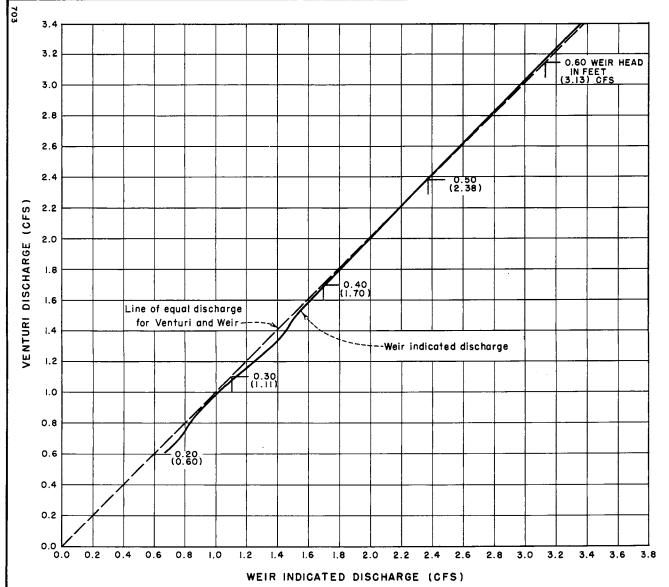
Flow Conditions for Weir Crest Elevations of 0.0 to 0.6 Foot at a Discharge of 1.4 cfs



ADJUSTABLE CIPOLLETTI WEIR STUDY

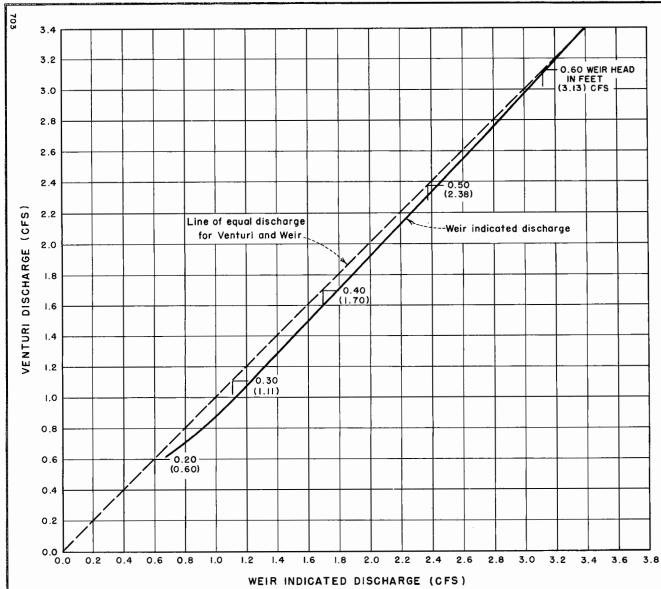
CAPACITY CURVES FOR
2'-0" WEIR CREST 0.10 FOOT OR HIGHER

ABOVE STATIONARY WEIR CREST



ADJUSTABLE CIPOLLETTI WEIR STUDY CAPACITY CURVES FOR

2'-0" WEIR CREST 0.05 FOOT ABOVE STATIONARY WEIR CREST



ADJUSTABLE CIPOLLETTI WEIR STUDY

CAPACITY CURVES FOR
2'-0" WEIR CREST 0.00 FOOT ABOVE

STATIONARY WEIR CREST

CONVERSION FACTORS-BRITISH TO METRIC UNITS OF MEASUREMENT

The following conversion factors adopted by the Bureau of Reclamation are those published by the American Society for Testing and Materials (ASTM Metric Practice Guide, January 1964) except that additional factors (*) commonly used in the Bureau have been added. Further discussion of definitions of quantities and units is given on pages 10-11 of the ASTM Metric Practice Guide.

The metric units and conversion factors adopted by the ASTM are based on the "International System of Units" (designated SI for Systeme International d'Unites), fixed by the International Committee for Weights and Measures; this system is also known as the Giorgi or MKSA (meter-kilogram (mass)-second-ampere) system. This system has been adopted by the International Organization for Standardization in ISO Recommendation R-31.

The metric technical unit of force is the kilogram-force; this is the force which, when applied to a body having a mass of 1 kg, gives it an acceleration of 9.80665 m/sec/sec, the standard acceleration of free fall toward the earth's center for sea level at 45 deg latitude. The metric unit of force in SI units is the newton (N), which is defined as that force which, when applied to a body having a mass of 1 kg, gives it an acceleration of 1 m/sec/sec. These units must be distinguished from the (inconstant) local weight of a body having a mass of 1 kg; that is, the weight of a body is that force with which a body is attracted to the earth and is equal to the mass of a body multiplied by the acceleration due to gravity. However, because it is general practice to use "pound" rather than the technically correct term "pound-force," the term "kilogram" (or derived mass unit) has been used in this guide instead of "kilogram-force" in expressing the conversion factors for forces. The newton unit of force will find increasing use, and is essential in SI units.

Table 1

	Table 1			
Q0	QUANTITIES AND UNITS OF SPACE			
Multiply	Ву	To obtain		
·	LENGTH			
Mil. Inches Feet Yards. Miles (statute)	. 1.609344 (exactly)	Millimeters Centimeters Centimeters Meters		
	AREA	·		
Square inches	929.03 (exactly)* 0.092903 (exactly) 0.836127 0.40469*	Square meters Square meters Hectares		
Square miles	2.58999			
	VOLUME			
Cubic inches	. 0.0283168			
	CAPACITY			
Fluid ounces (U.S.) Liquid pints (U.S.) Quarts (U.S.). Gallons (U.S.)	29.5729. 0.473179. 0.473166. 9,463.58. 0.946358. 3,785.43* 3.78543 3.78543 0.00378543*	Cubic decimeters Liters Cubic centimeters Liters Cubic centimeters Cubic decimeters Liters Liters Cubic meters		
Gallons (U.K.)	4.54596	Liters Liters Cubic meters		

C.

Table II

QUANTITIES	ΔND	IINTTS	OF	MECHANICS

Multiply	Ву	To obtain	Multiply	Ву	To obtain
	MASS		· · · · · · · · · · · · · · · · · · ·	FORCE*	
Grains (1/7,000 lb) Troy ounces (480 grains). Ounces (avdp)	31,1035	Grams	Pounds	0.453592*	
Pounds (avdp)	0.45359237 (exactly).	Kilograms		WORK AND ENERGY*	
		Metric tons		1,055.06	
	FORCE/AREA		Btu per pound		Joules per gram Joules
Pounds per square inch .		070307		POWER	
Pounds per square foot .	4.88243	Kilograms per square meter Newtons per square meter	Horsepower		Watts
	MASS/VOLUME (DENSITY)		1000-pounds per 600000	HEAT TRANSFER	
Ounces per cubic inch Pounds per cubic foot Tons (long) per cubic yard	16.0185 . 0.0160185 . 1.32894.		Btu in./hr ft ² deg F (k, thermal conductivity)	0.1240	Milliwatts/cm deg C Kg cel/hr m deg C Kg cel m/hr m ² deg C
	MASS/CAPACITY		Btu/hr ft2 deg F (C, thermal conductance)	0.568	Milliwatts/cm2 deg C
Ounces per gallon (U.S.). Ounces per gallon (U.K.). Pounds per gallon (U.S.). Pounds per gallon (U.K.).	6.2362		Deg F hr ft ² /Btu (R, thermal resistance)	4.882	Kg cal/hr m ² deg C Deg C cm ² /milliwatt J/g deg C
	BENDING MOMENT OR TORQUE		Btu/lb deg F	1.000*	Cal/gram deg C
Foot-pounds	1.12985 x 10 ⁶ 0.138255 1.35582 x 10 ⁷ 5.4431 72,008	Meter-kilograms Centimeter-dynes Meter-kilograms Centimeter-dynes	Grains/hr ft ² (water vapor trensmission). Perms (permeance). Perm-inches (permeability)	0.09290*	M ² /hr N Grams/24 hr m ²
Feet per second	VELOCITY 30.48 (exactly)	Centimeters per second			
Feet per year Miles per hour	0.3048 (exactly)* 0.965873 x 10-5*	Meters per second Centimeters per second	·	Table III OTHER QUANTITIES AND UNITS	S
		Meters per second	Multiply	Ву	To obtain
	ACCELERATION*		Cubic feet per square foot per day (seepage)	304 8 *	Liters per square meter per day
Feet per second2		Meters per second ²	Pound-seconds per square foot		
Cubic feet per second (sec feet)	0.028317*	Cubic meters per second Liters per second Liters per second	(viscosity)	0.02903* (exactly)	. Kilogram second per square meter . Square meters per second . Celsius or Kelvin degrees (change) Kilovolts per millimeter . Lumans per square meter . Ohm-square millimeters per meter . Millicuries per cubic meter . Milliamps per square meter
			Callons per square yard	4.527219*	Liters per equare meter Kilograms per centimeter

ABSTRACT

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VERTICAL ADJUSTABLE WEIR
Laboratory Report, Bureau of Reclamation, Denver, 13 p, including 8 fig, 1966

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