

# Inundation Modeling, Breach Parameters, and Consequences (Introduction)

Best Practices in Dam and Levee Safety Risk Analysis  
Part C – Consequence Estimating

Last modified June 2017, presented July 2019



US Army Corps  
of Engineers®



# Key Concepts

- Risk management involves consequence management
- Scalable approach based on goals of analysis
  - Initial characterization vs. prioritization vs. risk reduction
- Life risk is paramount
  - Understanding human factors is critical
- Build the case
  - How many people are exposed?
    - Warning and evacuation considerations
  - Flood characteristics?
    - Breach parameters
    - Inundation modeling
- Embrace uncertainty



# Definitions

- Consequence
  - Direct vs. Indirect
- Life Loss
  - Population at risk
  - Exposed/threatened population
  - Fatality rate
- Economic
- Environmental
- Cultural



# Essential Elements of Life Loss Estimate

- How many people are exposed to the flooding?
  - Initial distribution of people
  - Redistribution through evacuation
- How severe is the flooding?
- Are the people in a structure that can withstand the flooding?
- Will some of the people subjected to flooding die?



# Empirical vs. Simulation Models

## Empirical:

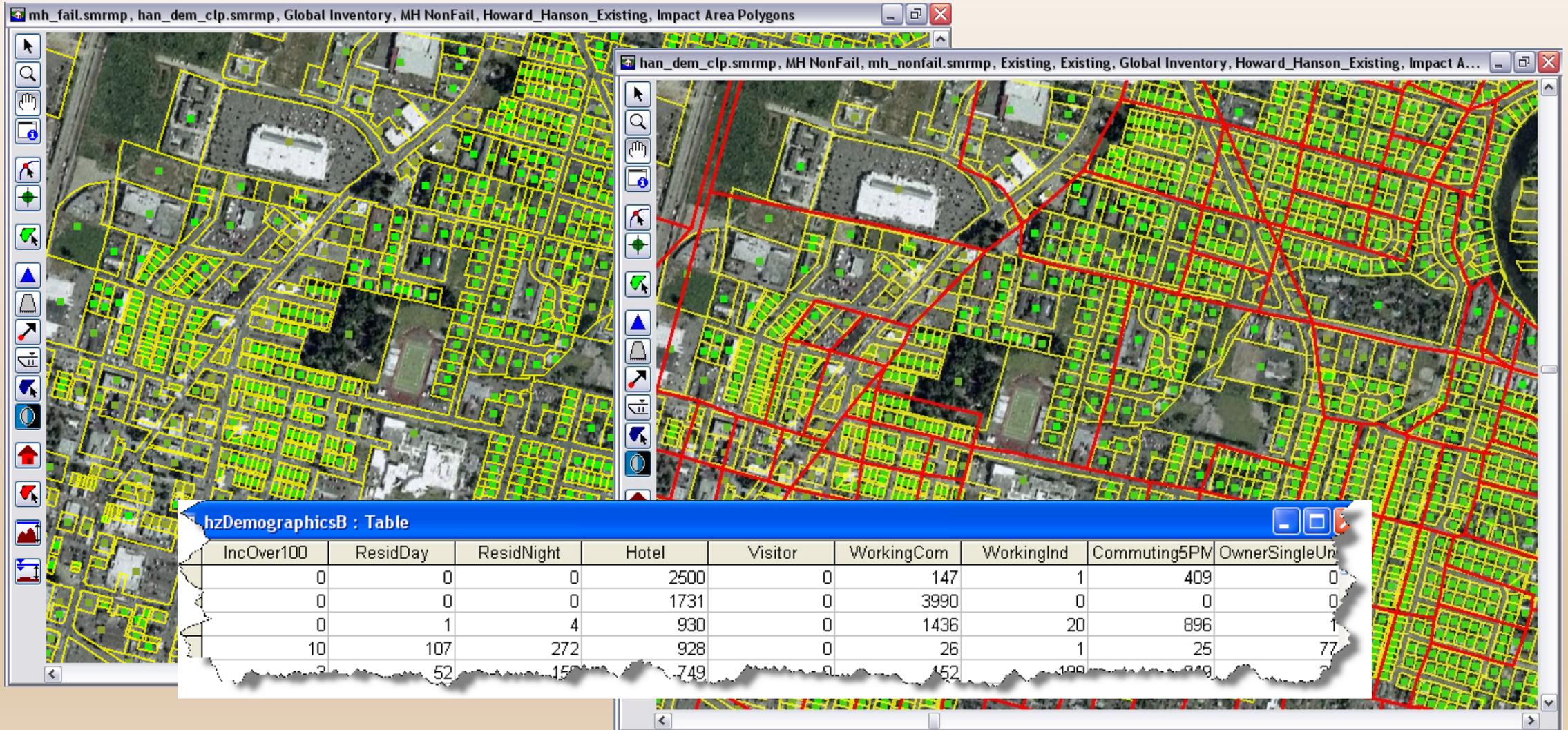
- Groups of PAR evaluated in aggregate
- Fatality rates ranges reflect evacuation rate assumptions – evacuation is not explicitly modeled
- Relevant parameters are warning time and the intensity of flooding

## Simulation:

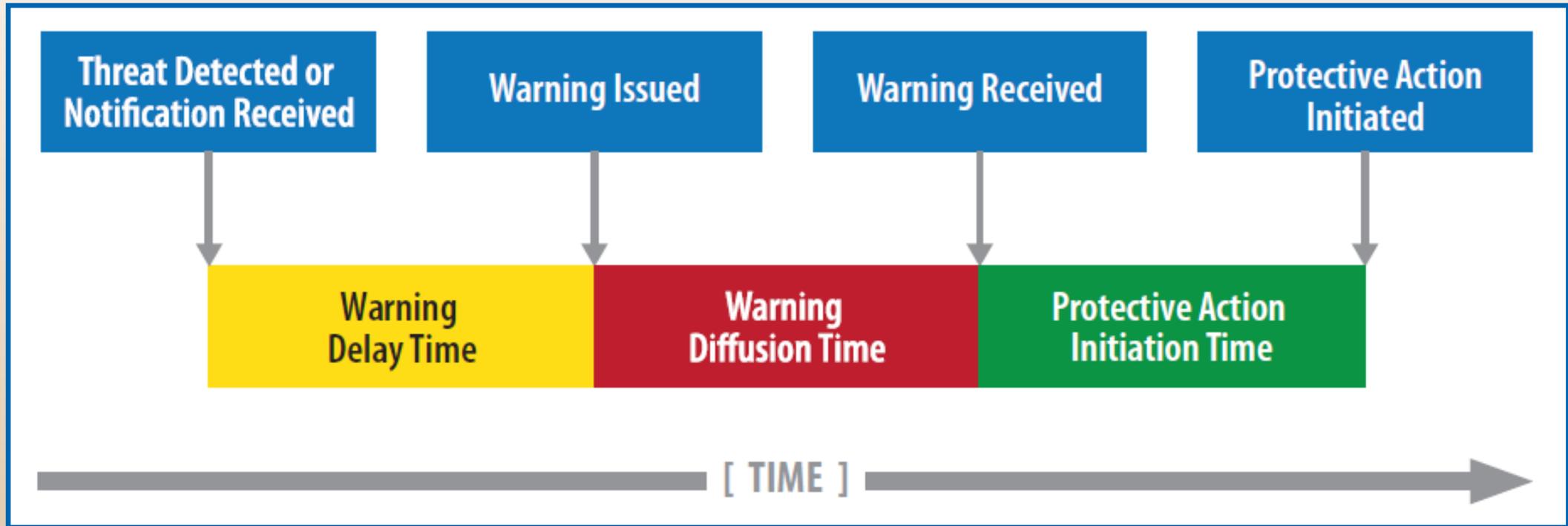
- Tracks movement of people and movement of water – evacuation is explicitly modeled
- Each individual or defined group is evaluated separately
- Fatality rates can be applied to PAR which exceed critical flood parameter thresholds

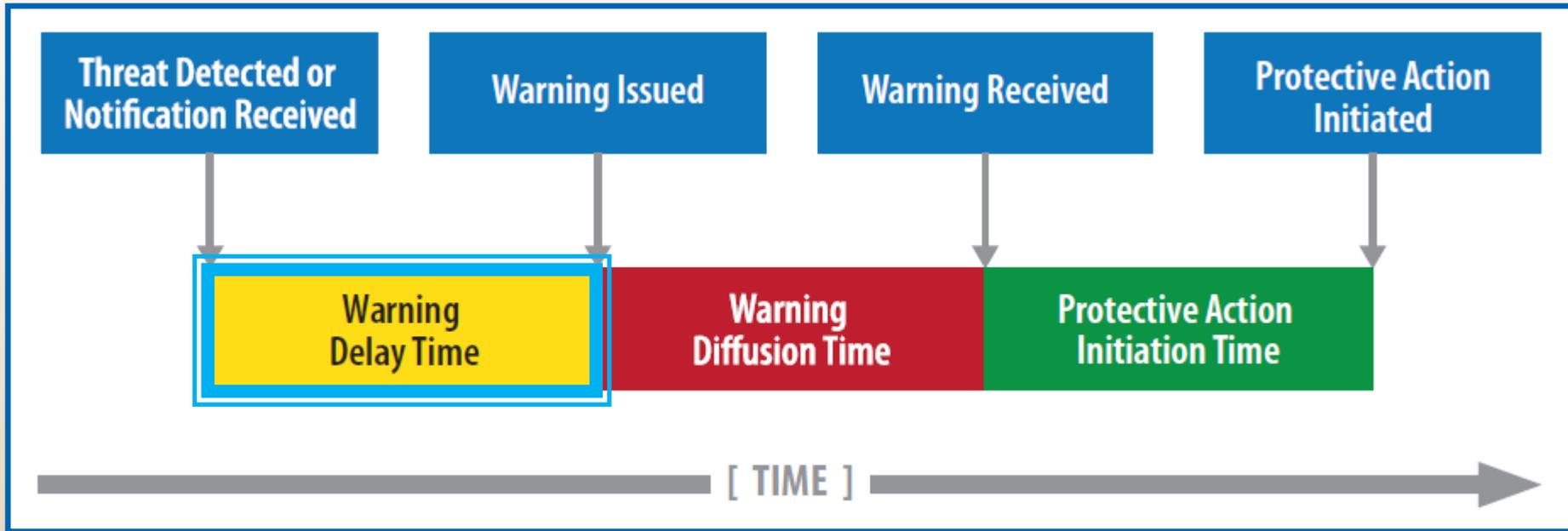


# Initial Distribution of People

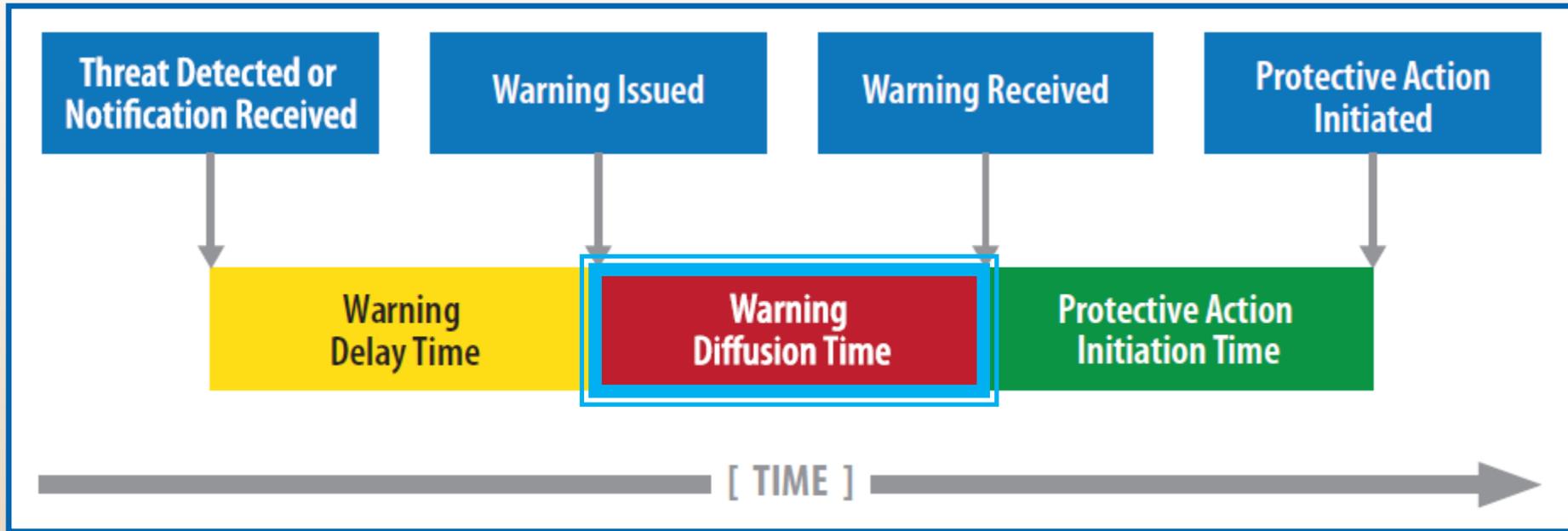


# Redistribution of People (Evacuation Effectiveness)

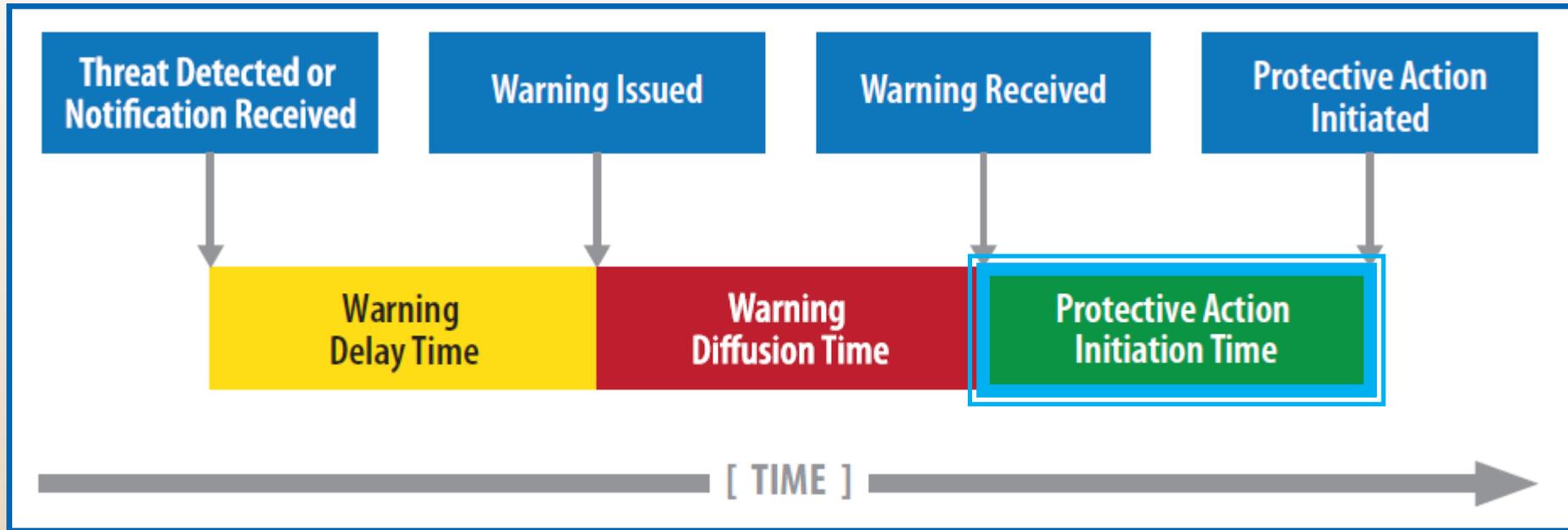




- Standard Warning Plan and Standard Operating Procedures are *Written Down*
- Warning Thresholds Are in Place
- SOP Drills Are Conducted
- Responsibilities are Identified and Clearly Define Authority To Issue Warnings



- Number and mix of warning channels
- Frequency of distribution
- Ability to wake people up
- Modern technologies



- Message content and style

# Message Content

- **The single most important thing that an emergency manager can do** to motivate effective public protective action is to provide the best emergency messages possible.

**SOURCE:** say who the message

**THREAT:** describe the flooding

**LOCATION:** state the impact area

**GUIDANCE/TIME:** tell people

**EXPIRATION TIME:** tell people when the alert/warning expires and/or new information will be received



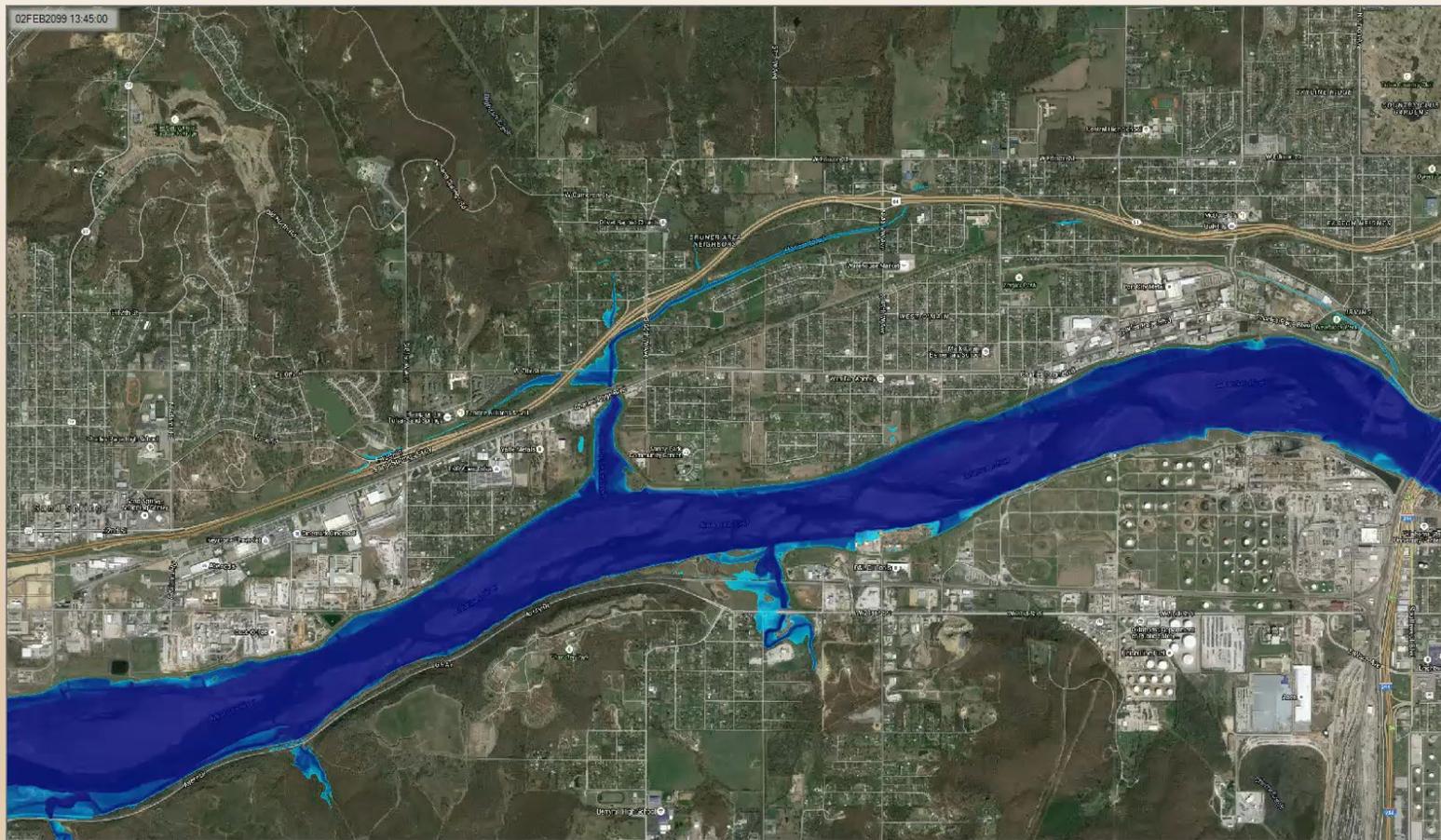
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# Flood Severity

- Depth
- Velocity
- Depth \* Velocity
- Arrival time
- Extents



# Key Concepts For Inundation Modeling

- Scenario
  - Pool or stage elevation and hydrology
  - Breach or Non-breach
  - Failure mode
- Breach parameters
- Terrain
  - 1d vs. 2d
- Initial conditions
- Incremental/coincident flows

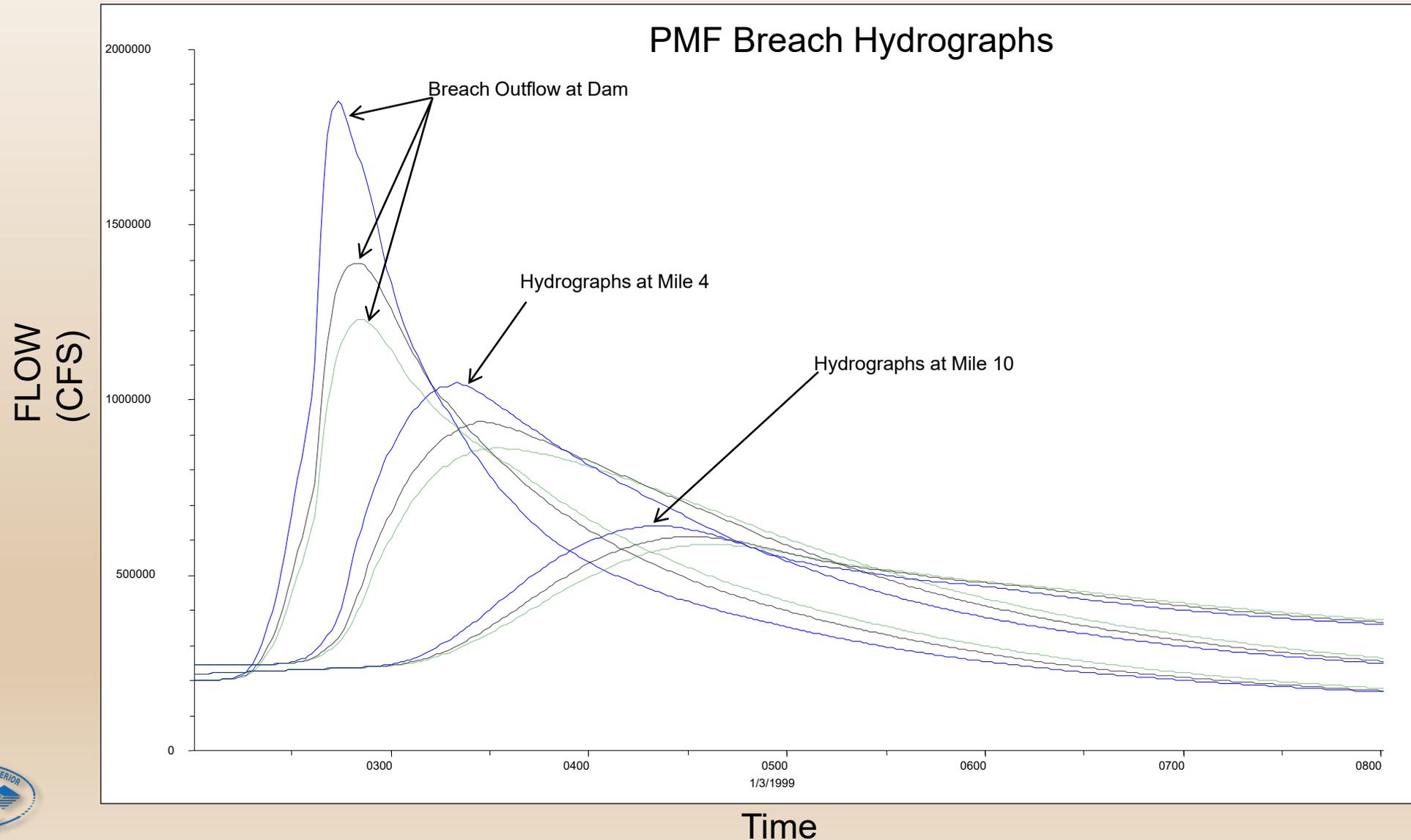


# Key Concepts for Understanding and Selecting Breach Parameters

- Breach parameters can impact the following flood characteristics
  - Depth
  - Velocity
  - Arrival time (and therefore warning time)
  - Consequences
    - Life loss, direct damage, repair costs, etc
- Sensitivity analysis should be performed prior to detailed breach parameter analysis
  - Adopt scalable approach based on outcome
- Tradition empirical equations are based on dam breach cases

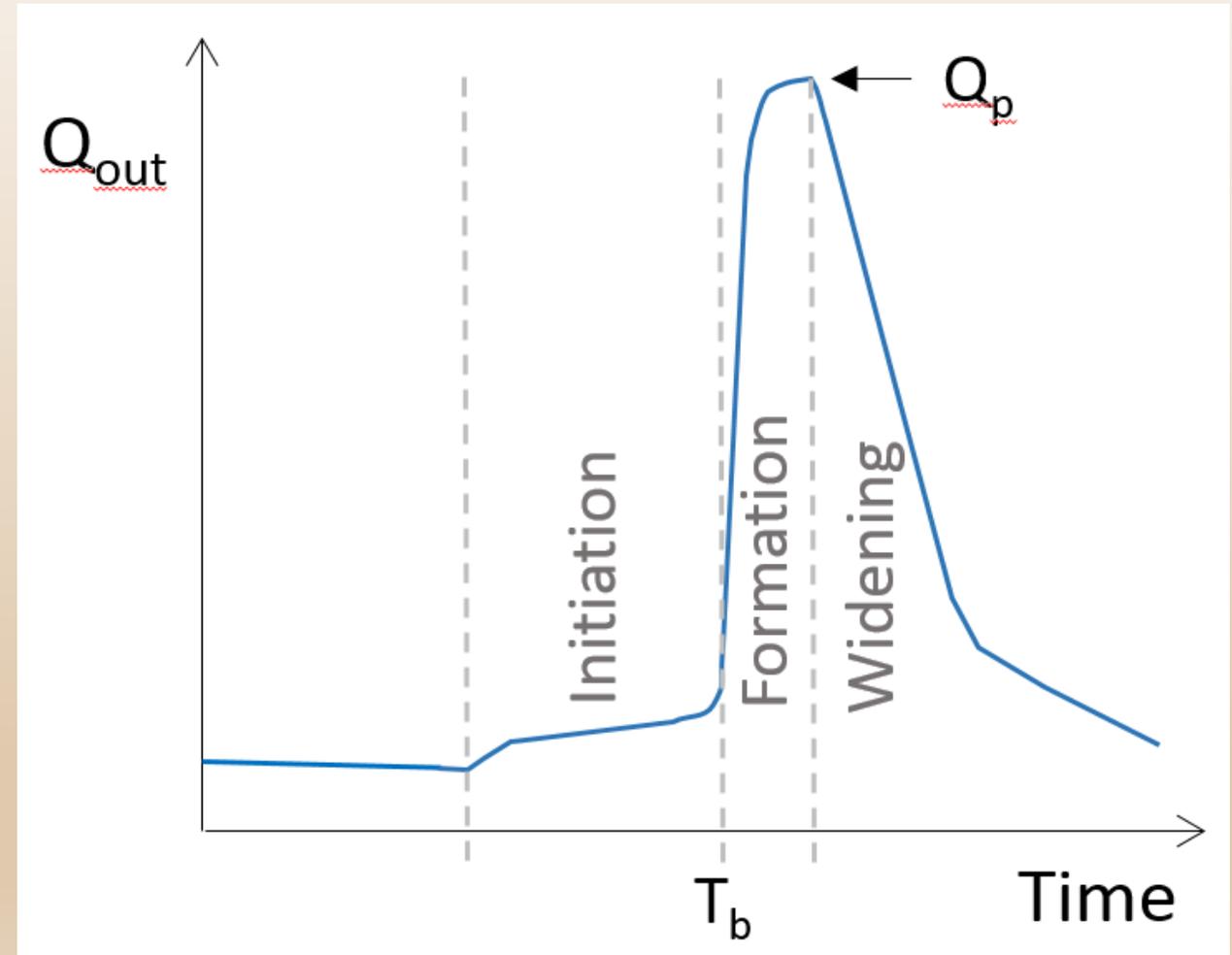


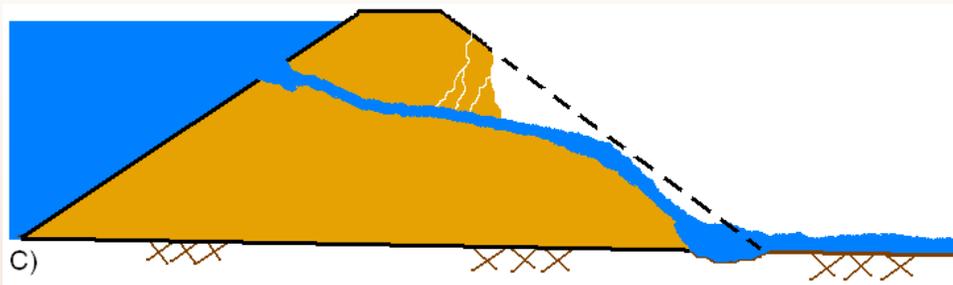
# Does it Matter? Depends on downstream terrain, location of PAR and other factors..



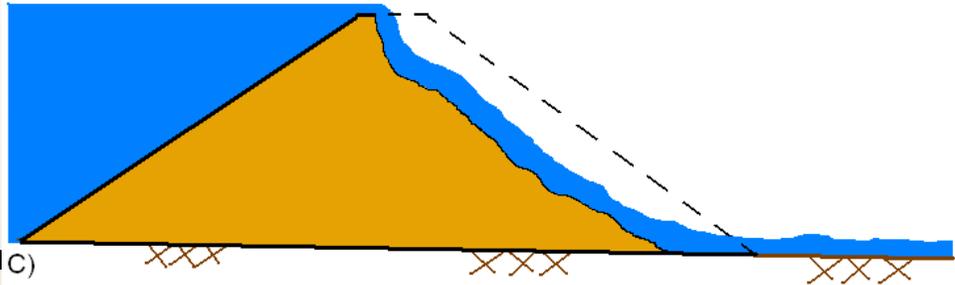
# Breach Parameters Definitions

- Breach initiation
  - Typically not included in hydraulic model
- Time of breach ( $T_b$ )
- Breach formation
- Breach widening



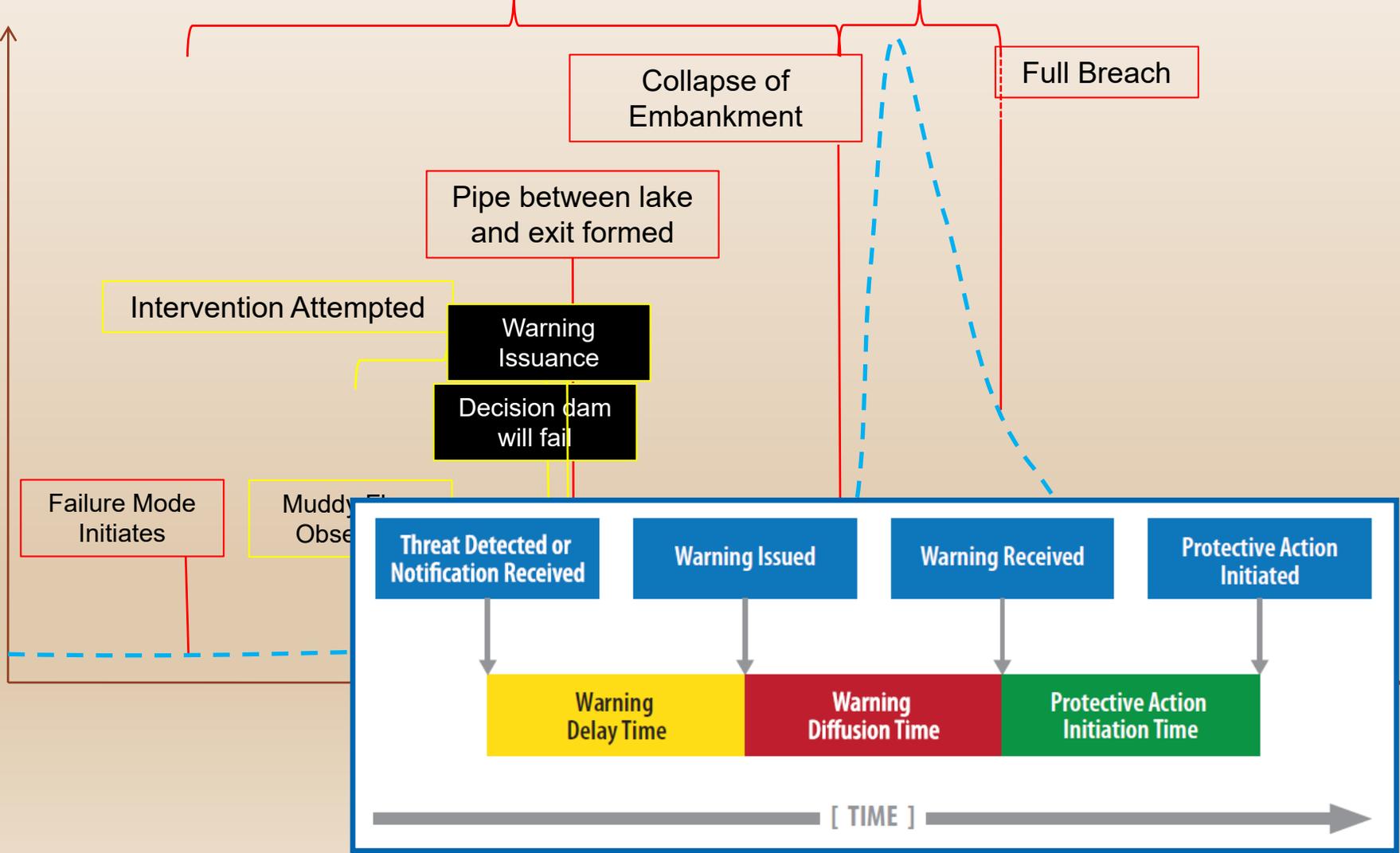


Initiation Time



Breach Flow (MacDonald, Froemich, v. & G)

Flow



# Options for Estimating Breach Parameters

- User defined
  - Historic data, empirical equations, site specific assumptions, etc
- Simplified physical breaching
  - Velocity vs. erosion rate
- Coupled embankment erosion and hydraulic model

Reference	Number of Case Studies	Relations Proposed (S.I. units, meters, m <sup>3</sup> /s, hours)
Johnson and Illes (1976)		$0.5h_d \leq B \leq 3h_d$ for earthfill dams
Singh and Snorrason (1982, 1984)	20	$2h_d \leq B \leq 5h_d$ $0.15 \text{ m} \leq d_{\text{outtop}} \leq 0.61 \text{ m}$ $0.25 \text{ hr} \leq t_f \leq 1.0 \text{ hr}$
MacDonald and Langridge-Monopolis (1984)	42	Earthfill dams: $V_{er} = 0.0261(V_{out}^* h_w)^{0.769}$ [best-fit] $t_f = 0.0179(V_{er})^{0.364}$ [upper envelope] Non-earthfill dams: $V_{er} = 0.00348(V_{out}^* h_w)^{0.852}$ [best fit]
FERC (1987)		$B$ is normally 2-4 times $h_d$ $B$ can range from 1-5 times $h_d$ $Z = 0.25$ to $1.0$ [engineered, compacted dams] $Z = 1$ to $2$ [non-engineered, slag or refuse dams] $t_f = 0.1$ - $1$ hours [engineered, compacted earth dam] $t_f = 0.1$ - $0.5$ hours [non-engineered, poorly compacted]
Froehlich (1987)	43	$\bar{B}^* = 0.47 K_o (S^*)^{0.25}$ $K_o = 1.4$ overtopping; $1.0$ otherwise $Z = 0.75 K_c (h_w^*)^{1.57} (\bar{W}^*)^{0.73}$ $K_c = 0.6$ with corewall; $1.0$ without a corewall $t_f^* = 79(S^*)^{0.47}$
Reclamation (1988)		$B = (3)h_w$ $t_f = (0.011)B$
Singh and Scarlatos (1988)	52	Breach geometry and time of failure tendencies $B_{\text{top}}/B_{\text{bottom}}$ averages 1.29
Von Thun and Gillette (1990)	57	$B, Z, t_f$ guidance (see discussion)
Dewey and Gillette (1993)	57	Breach initiation model; $B, Z, t_f$ guidance
Froehlich (1995b)	63	$\bar{B} = 0.1803 K_o V_w^{0.32} h_b^{0.19}$ $t_f = 0.00254 V_w^{0.53} h_b^{(-0.90)}$ $K_o = 1.4$ for overtopping; $1.0$ otherwise



# Numeric Modeling Options for Estimating Breach Parameters

- User defined
  - Historic data, empirical equations, site specific assumptions, etc
- Simplified physical breaching
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Process	WinDAM B/C	DL Breach	HR BREACH	NWS BREACH
River Hydraulics	No	N	N	N
Breach Flow	Yes	Y	Y	Y
Internal Hydraulic Routing	N	N	Y	N
Tailwater Submergence	Y	Y	Y	Y
Piping Initiated	Y	Y	Y	Y
Overtopping Initiated	Y	Y	Y	Y
River Erosion and Stability Failure Initiated	N	N	N	N
Headcut	Y	Y	Y	N
Breach Widening	Y	Y	Y	Y
Breach Deepening	Y	Y	Y	Y
Foundation Scouring	N	Y	N	N
Mass Wasting	Y	Y	Y	Y
Surface Erosion by Sediment Transport	N	Y	Y	Y
Sediment Volume	N	Y	Y	Y
Surface Protection Removal	Y	N	Y	Y
Composite Material Zones	N	Y	Y	Y



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Building type	Partial damage	Total damage
<b>Wood-framed</b>		
<b>unanchored</b>	$v*d \geq 2 \text{ m}^2/\text{s}$	$v*d \geq 3 \text{ m}^2/\text{s}$
<b>anchored</b>	$v*d \geq 3 \text{ m}^2/\text{s}$	$v*d \geq 7 \text{ m}^2/\text{s}$
<b>Masonry, concrete &amp; brick</b>	$v \geq 2 \text{ m/s}$ & $v*d \geq 3 \text{ m}^2/\text{s}$	$v \geq 2 \text{ m/s}$ & $v*d \geq 7 \text{ m}^2/\text{s}$

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Ranges of fatality rates and life loss estimates are required for the empirical approach

# Embrace Uncertainty

