

Quantity Estimate Supporting Documents

Quantity Estimation Spread Sheets

The quantity estimation spread sheets were developed based upon the following four (5) input parameters:

- H_E : Embankment Height (ft)
- H_F : Foundation Height (ft)
- H_S : Seafloor Deposits Thickness (ft)
- N_1 : Number of Stone Column at Upstream or Downstream Slope
- L : Length of Embankment

The embankment height H_E refers to the vertical distance between the dam crest and the top of the soft lacustrine/upper alluvium strata. The foundation height H_F refers to the thickness of the soft lacustrine/upper alluvium strata underneath the embankment.

Based upon these parameters, and the typical dam cross sections, the area and/or linear footage calculation equations for various embankment items were developed. These equations are presented in this part of this appendix. The quantity estimation spread sheets presented in the earlier part of this appendix were established based upon these equations.

The following Table A summarizes the required input parameters for calculating the quantity for each of the embankment items.

Table A – Required Input Parameters For Each Embankment Items

No.	Item	Unit	Input Parameters Required
1	Dredge of Seafloor Deposits	CY	L, H_E, H_S
1	Dredge of Soft Lacustrine/Upper Alluvium	CY	L, H_E, H_F
2	Sand/Gravel Fill Type A in Core	CY	L, H_E, H_F
2	Sand/Gravel Fill Type A in Shell	CY	L, H_E
3	Sand/Gravel Fill Type B in Shell	CY	L, H_E
5	Rip Rap	CY	L, H_E
6	Stone Column	FT	L, H_E, H_F, N_1
7	Installation of SCB Wall	SF	L, H_E, H_F
4	Sand Blanket Drain (Type A)	CY	L, H_E
3	Coarse Blanket Drain (Type B)	CY	L, H_E
8	Wick Drains	FT	L, H_E, H_F
9	Fine Rock Fill	CY	L, H_E

The input values for H_E , H_F and H_S are related to proposed dam crest elevations as well as interpretation of subsurface information such as the thickness of the seafloor deposits and the soft lacustrine/upper alluvium deposits. For the cost

estimation purposes, the subsurface data presented on the geologic cross sections A-A' through E-E' (Figure No. 9 through 13) in the Salton Sea Restoration Project Report prepared by URS were utilized.

Based on the assumed thickness of the seafloor and the soft lacustrine/upper alluvium deposits, the entire length of the proposed embankment was in several cases, divided into multiple sections for quantity estimation calculations. The following Table B summarizes the each section name and corresponding length for the various embankment dam options. The length of each individual section was measured on the maps provided by USBR.

Table B – Section Name and Length for Various Embankment Dam Options

	Section A (ft)	Section B (ft)	Section C (ft)	Section D (ft)	Section E (ft)
Mid Sea Dam	17672	8664	4854	7841	2297
Mid Sea Barrier	17969	6761	5534	5460	2366
South Sea Dam	74310				
North Sea Dam	122277				
Perimeter Dike	62555	29586			
Concentric Ring Dike	1327920				

A. Subsurface Data

PROJECT Saltun Sea PROJECT NO. 71600
 SUBJECT Subsurface Condition Summary BY PAA DATE 8/23/06
 REVIEWED BY JIE YH DATE 8/30/06

PURPOSE: Determine the average thickness of the seafloor deposits and the soft lacustrine and the alluvium deposits at different seafloor elevations based on CPT and borings data.

MIDSEA DAM

up to elevation -270

Based on B-5, B-26, B-4, CPT-28, CPT-3

<u>SOURCE</u>	<u>SEAFLOOR DEPOSITS THICKNESS (FEET)</u>	<u>SOFT LACUSTRINE THICKNESS (FEET)</u>	<u>UPPER ALLUVIUM THICKNESS (FEET)</u>
B-5	13'	13'	5'
B-26	15.5	23.5	4.5
B-4	21.5	16.5	0
CPT-28	14	25	0
CPT-3	<u>15</u>	<u>26</u>	<u>0</u>
AVG	15.8	20.8	1.9

Average thickness of seafloor deposits: 15.8'
 soft lacustrine/alluvium: 22.7'

elevations -260 to -270 (West)

Based on B-7, CPT-29, CPT-8

<u>SOURCE</u>	<u>SEAFLOOR DEPOSITS THICKNESS (FEET)</u>	<u>SOFT LACUSTRINE THICKNESS (FEET)</u>	<u>UPPER ALLUVIUM THICKNESS (FEET)</u>
B-7	3	5.5	23
CPT-29	11.8	10.6	5.9
CPT-8	<u>5.9</u>	<u>6.5</u>	<u>18.2</u>
AVG	6.9	7.5	15.7

6.1

Average thickness of seafloor deposits: 6.9'
 soft lacustrine/alluvium: 23.2'

PROJECT Saltier Sea PROJECT NO. 71100
 SUBJECT Subsurface Condition Summary BY RTH DATE 9/23/06
 REVIEWED BY JTE yu DATE 8/30/06

elevations -260 to -270 (EAST)
 Based on B-2, CPT-27

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
B-2	2	6	0
CPT-27	<u>11.8</u>	<u>18.8</u>	<u>0</u>
AVG	6.9	12.4	0

Average thickness of seafloor deposits: 6.9'
 soft lacustrine/alluvium: 12.4'

elevations -240 to -260 (WEST)
 Based on CPT-9, B-7

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
B-7	3	5.5	23
CPT-9	<u>3.5</u>	<u>0</u>	<u>17.6</u>
AVG	3.3	2.8	20.3

Average thickness of seafloor deposits: 3.3'
 soft lacustrine/alluvium: 23'

elevations -240 to -260 (EAST)
 Based on B-2, CPT-1

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
B-2	2	6	0
CPT-1	<u>6</u>	<u>5</u>	<u>0</u>
AVG	4	5.5	0

Average thickness of seafloor deposits: 4'
 soft lacustrine/alluvium: 5.5'

PERIMETER DIKE

Based on CPT-10, DH-1, B-11, CPT-12

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
CPT-10	3.6	1.2	14.8
DH-1	0	5.6	17.6
B-11	2.4	3.2	13.2
CPT-12	<u>4.4</u>	<u>0</u>	<u>9.2</u>
AVG	2.6	2.5	13.7

Average thickness of seafloor deposits: 2.6'
 soft lacustrine/alluvium: 16.2'; see notes on page 3

PROJECT Saltan Sea PROJECT NO. 71100
 SUBJECT Subsurface Condition Summary BY RAA DATE 9/23/06
 REVIEWED BY JIE YH DATE 8/30/06

NORTH SEA DAM

Based on B-20, CPT-23

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
B-20	3.75 (5)	5.6 (7.5)	3.8 (3.8)
260 CPT-23	<u>2</u> (5)	<u>0</u> (6)	<u>24</u> (15)
AVG	2.9 (5)	2.8 (6.8)	13.9 (9.4)

Average thickness of seafloor deposits: 2.9' (5)
 Soft lacustrine/alluvium: 16.7' (16.2)

SOUTH SEA DAM

Based on CPT-15, DH-2, B-14, CPT-13

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
CPT-15	2.5	7.5	2.5
250 DH-2	0	12	0
B-14	0	7.5	6.25
CPT-13	<u>0</u>	<u>0</u>	<u>8.75</u>
AVG	0.625	6.75	4.4

Average thickness of seafloor deposits: 0.6'
 Soft lacustrine/alluvium: 11.1' (13.8)

NOTE:

For South Sea Dam: use subsurface conditions at Elevation -250 in Section C-C' as well as

Section B-B'

C-C', near B-11	4'	1'	10'
B-B', near B-14	<u>0</u>	<u>8.5'</u>	<u>6.5'</u>
AVE:	2'	4.75	8.25'

Average Seafloor deposits = 2'

Average Soft Lacustrine/Alluvium = 13'

For perimeter Dike

use subsurface conditions at Elevation -250 in Section C-C' as well as Section A-A'

C-C', near CPT-10.	2.5'	2'	15'
A-A', near CPT-10	<u>4.1'</u>	<u>0.6'</u>	<u>18.8'</u>

Average Seafloor Deposits = 3.3'; Average Soft Lacustrine/Alluvium = 18.2' 16.9'

PROJECT Saltan Sea PROJECT NO. 71100
 SUBJECT Subsurface Condition Summary BY RAA DATE 8/25/06
 REVIEWED BY JIE YU DATE 8/30/06

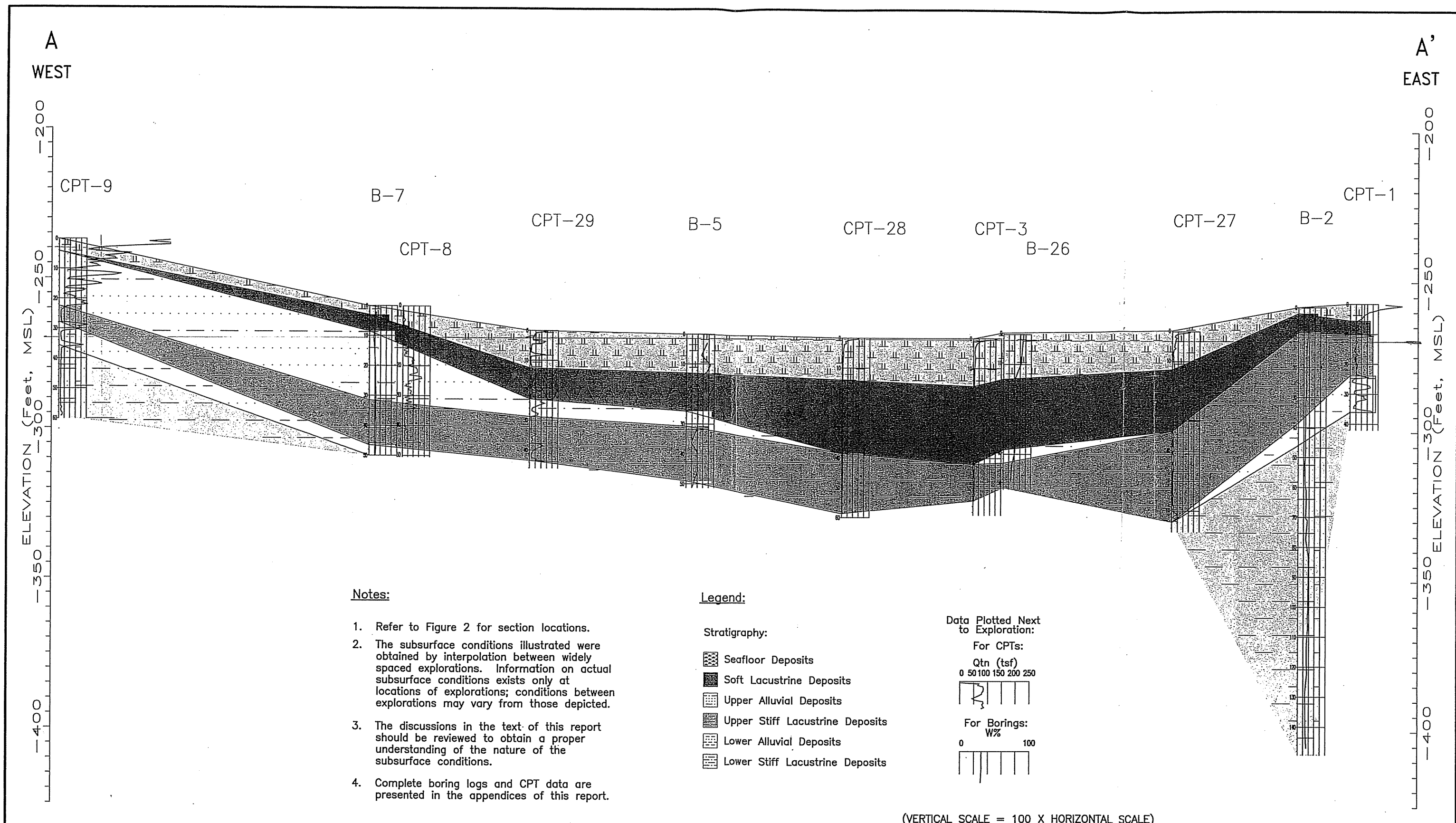
Concentric Rings

Based on B-5, B-26, B-4, B-7, B-2, B-11, B-20, B-14, B-19, B-6, B-17, B-26⁺
 CPT-28, CPT-3, CPT-29, CPT-8, CPT-27, CPT-1, CPT-10, CPT-12⁺
 CPT-23, CPT-15, CPT-13, CPT-22, CPT-18, CPT-16, CPT-9, CPT-3⁺
 DH-1, DH-2, DH-3

SOURCE	SEAFLOOR DEPOSITS THICKNESS (FEET)	SOFT LACUSTRINE THICKNESS (FEET)	UPPER ALLUVIUM THICKNESS (FEET)
B-5	1.3	1.3	5
B-26	15.5	23.5	4.5
B-4	21.5	16.5	10
B-7	3	5.5	2.3
B-2	2	3.6	0
B-11	2.4	3.2	13.2
B-20	3.75	5.6	3.2
B-14	6	5.5	5
B-6	12	14.5	60
B-17	10	2.4	60
B-19	15.5	14	46
B-26	15.5	23.5	4.5
CPT-28	14	25	0
CPT-3	11.8	26	0
CPT-29	11.8	10.6	5.9
CPT-8	11.8	6.5	18.2
CPT-27	11.8	18.8	0
CPT-1	3.6	5.8	14.8
CPT-10	4.6	1.2	9.2
CPT-12	4.4	0	2.4
CPT-23	2	0	2.4
CPT-15	2.5	7.5	8.8
CPT-13	20	0	8.8
CPT-22	20	14	10
CPT-18	7.2	1.6	0
CPT-16	7.2	4	4.8
CPT-9	3.9	0	17.6
CPT-3	14.1	23.5	2.4
DH-1	0	5.6	17.6
DH-2	0	12	0
DH-3	0	4.8	0
AVG	7.8	9.3	6.3

Average thickness of

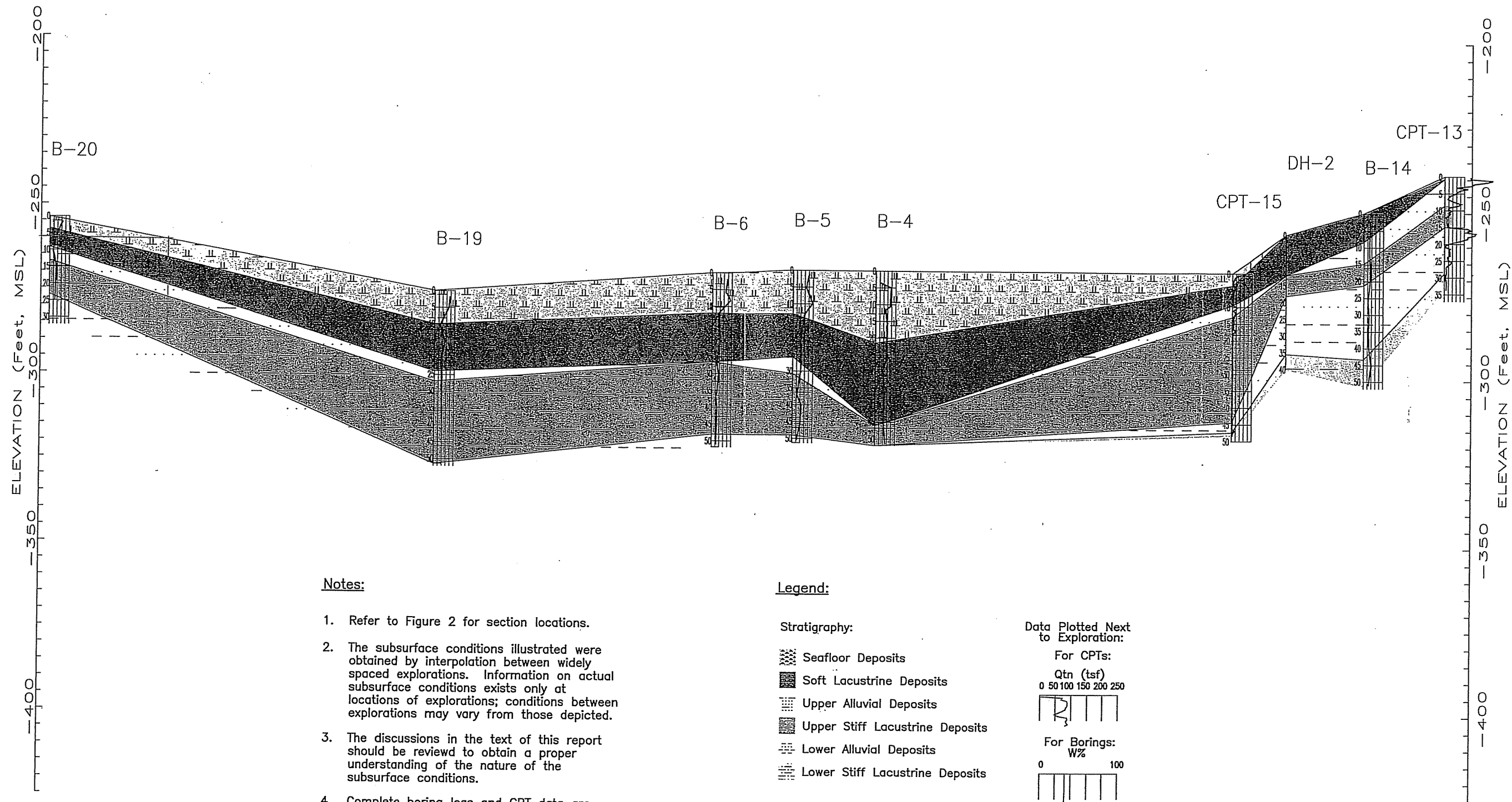
Seafloor deposits: 7.8'
 Soft lacustrine/alluvium: 15.6'



GEOLOGIC CROSS SECTION A-A'			
SALTON SEA RESTORATION PROJECT			
	1500 0 1500 3000 Feet	CHECKED BY: AG	DATE: 1-29-04
	SCALE: 1" = 3000'	PM: LDH	PROJ. NO: 27663042.00005
			FIG. NO: 9

B
NORTHWEST

B'
SOUTHEAST



Notes:

1. Refer to Figure 2 for section locations.
2. The subsurface conditions illustrated were obtained by interpolation between widely spaced explorations. Information on actual subsurface conditions exists only at locations of explorations; conditions between explorations may vary from those depicted.
3. The discussions in the text of this report should be reviewed to obtain a proper understanding of the nature of the subsurface conditions.
4. Complete boring logs and CPT data are presented in the appendices of this report.

Legend:

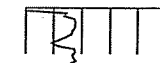
Stratigraphy:

- Seafloor Deposits
- Soft Lacustrine Deposits
- Upper Alluvial Deposits
- Upper Stiff Lacustrine Deposits
- Lower Alluvial Deposits
- Lower Stiff Lacustrine Deposits

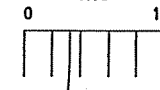
Data Plotted Next to Exploration:

For CPTs:

Qtn (tsf)
0 50 100 150 200 250



For Borings:
W%



(VERTICAL SCALE = 400 X HORIZONTAL SCALE)

GEOLOGIC CROSS SECTION B-B'
SALTON SEA RESTORATION PROJECT

URS

6250 0 6250 12500 Feet
SCALE: 1" = 12500'

CHECKED BY: AG
PM: LDH

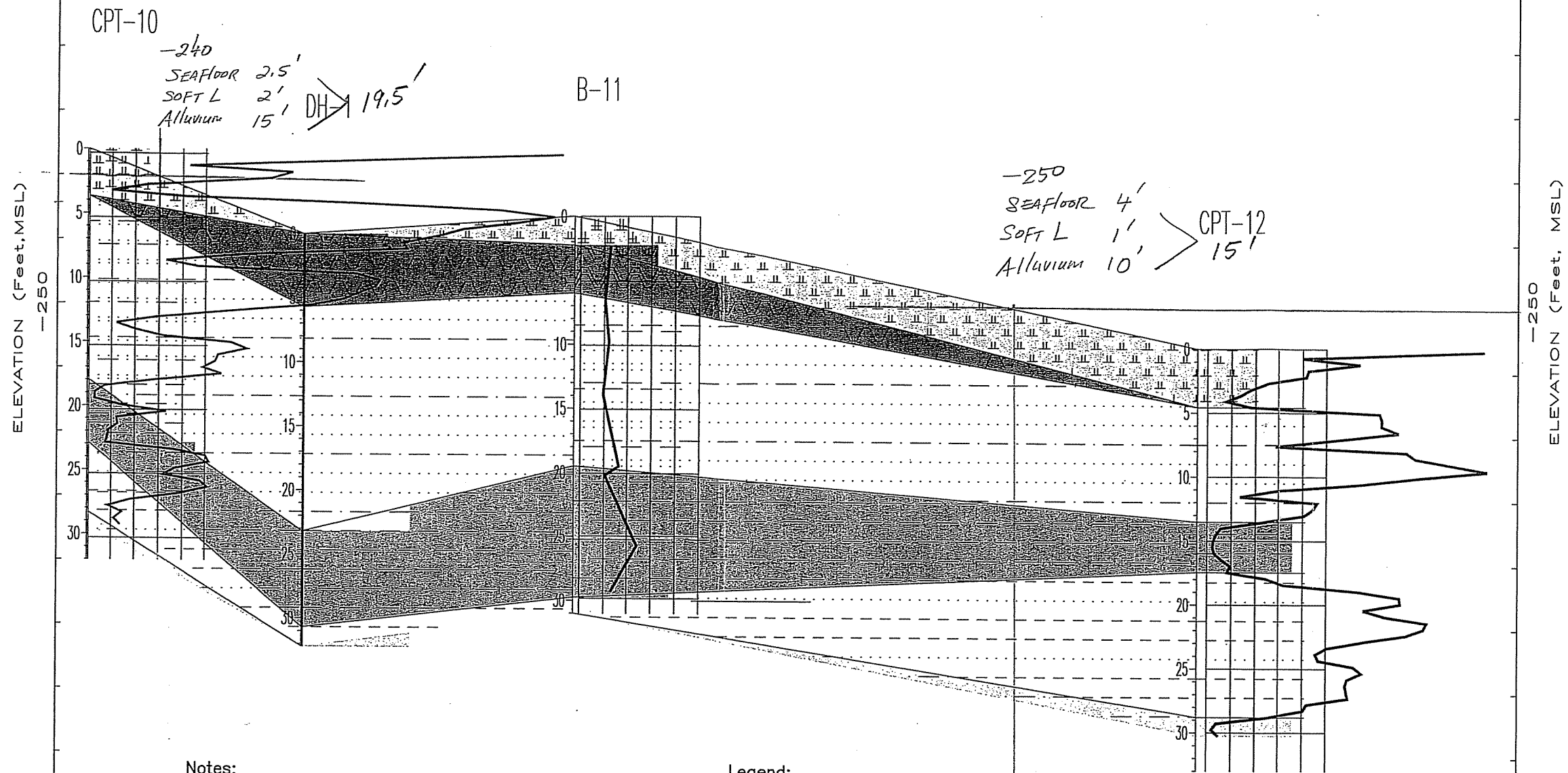
DATE: 12-10-03
PROJ. NO: 27663042.00005

FIG. NO:
10

C
WEST

C'
EAST

1" = 10'



Notes:

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3. The discussions in the text of this report should be reviewed to obtain a proper understanding of the nature of the subsurface conditions.
4. Complete boring logs and CPT data are presented in the appendices of this report.

Legend:

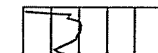
Stratigraphy:

- Seafloor Deposits
- Soft Lacustrine Deposits
- Upper Alluvial Deposits
- Upper Stiff Lacustrine Deposits
- Lower Alluvial Deposits
- Lower Stiff Lacustrine Deposits

Data Plotted Next to Exploration:

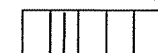
For CPTs:

Qtn (tsf)
0 50 100 150 200 250



For Borings:

W%
0 100



(VERTICAL SCALE = 100 X HORIZONTAL SCALE)

GEOLOGIC CROSS SECTION C-C'
SALTON SEA RESTORATION PROJECT

URS

500 0 500 1000 Feet
SCALE: 1" = 1000'

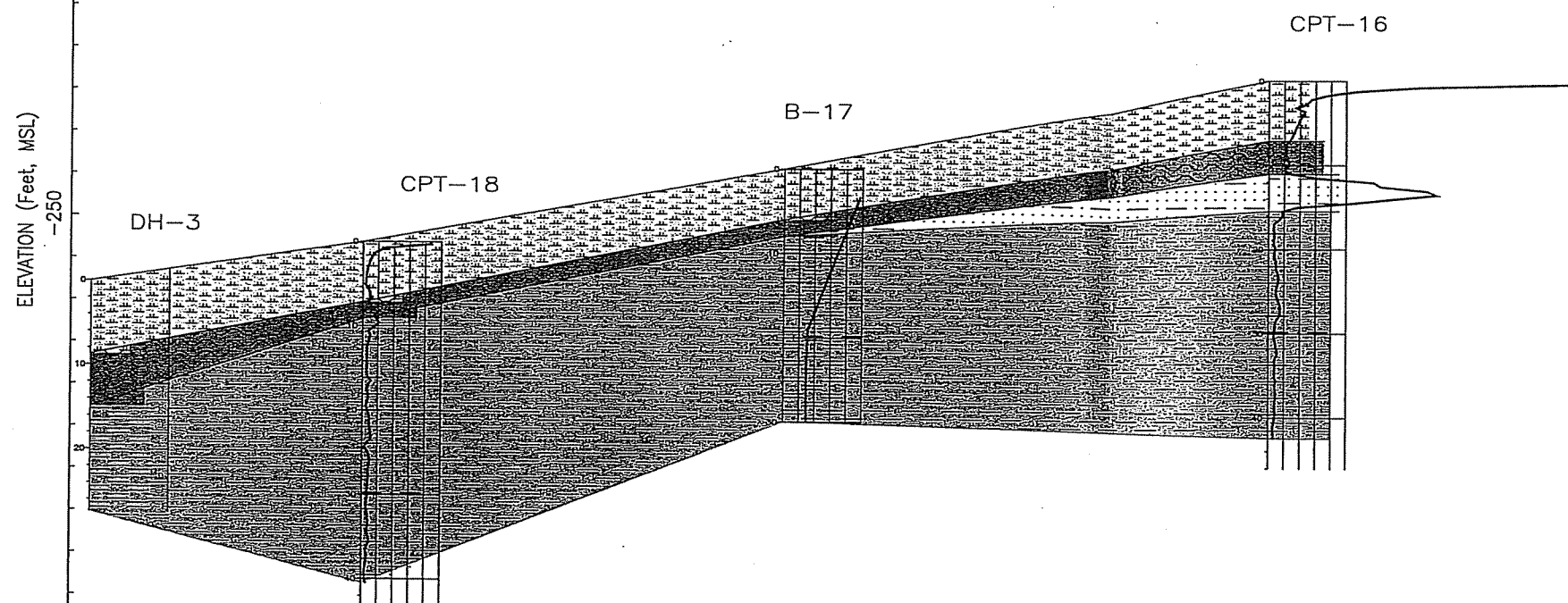
CHECKED BY: AG
PM: LDH

DATE: 1-29-04
PROJ. NO: 27663042.00005

FIG. NO:
11

D
WEST
-200
-250
-300
ELEVATION (Feet, MSL)

D'
EAST
-200
-250
-300
ELEVATION (Feet, MSL)



Legend:

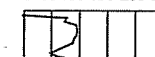
Stratigraphy:

- Seafloor Deposits
- Soft Lacustrine Deposits
- Upper Alluvial Deposits
- Upper Stiff Lacustrine Deposits
- Lower Alluvial Deposits
- Lower Stiff Lacustrine Deposits

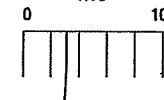
Data Plotted Next to Exploration:

For CPTs:

Qtn (tsf)
0 50 100 150 200 250



For Borings:
W%



Notes:

1. Refer to Figure 2 for section locations.
2. The subsurface conditions illustrated were obtained by interpolation between widely spaced explorations. Information on actual subsurface conditions exists only at locations of explorations; conditions between explorations may vary from those depicted.
3. The discussions in the text of this report should be reviewed to obtain a proper understanding of the nature of the subsurface conditions.
4. Complete boring logs and CPT data are presented in the appendices of this report.

(VERTICAL SCALE = 100 X HORIZONTAL SCALE)

GEOLOGIC CROSS SECTION D-D'
SALTON SEA RESTORATION PROJECT

URS

1000 0 1000 2000 Feet
SCALE: 1" = 2000'

CHECKED BY: AG

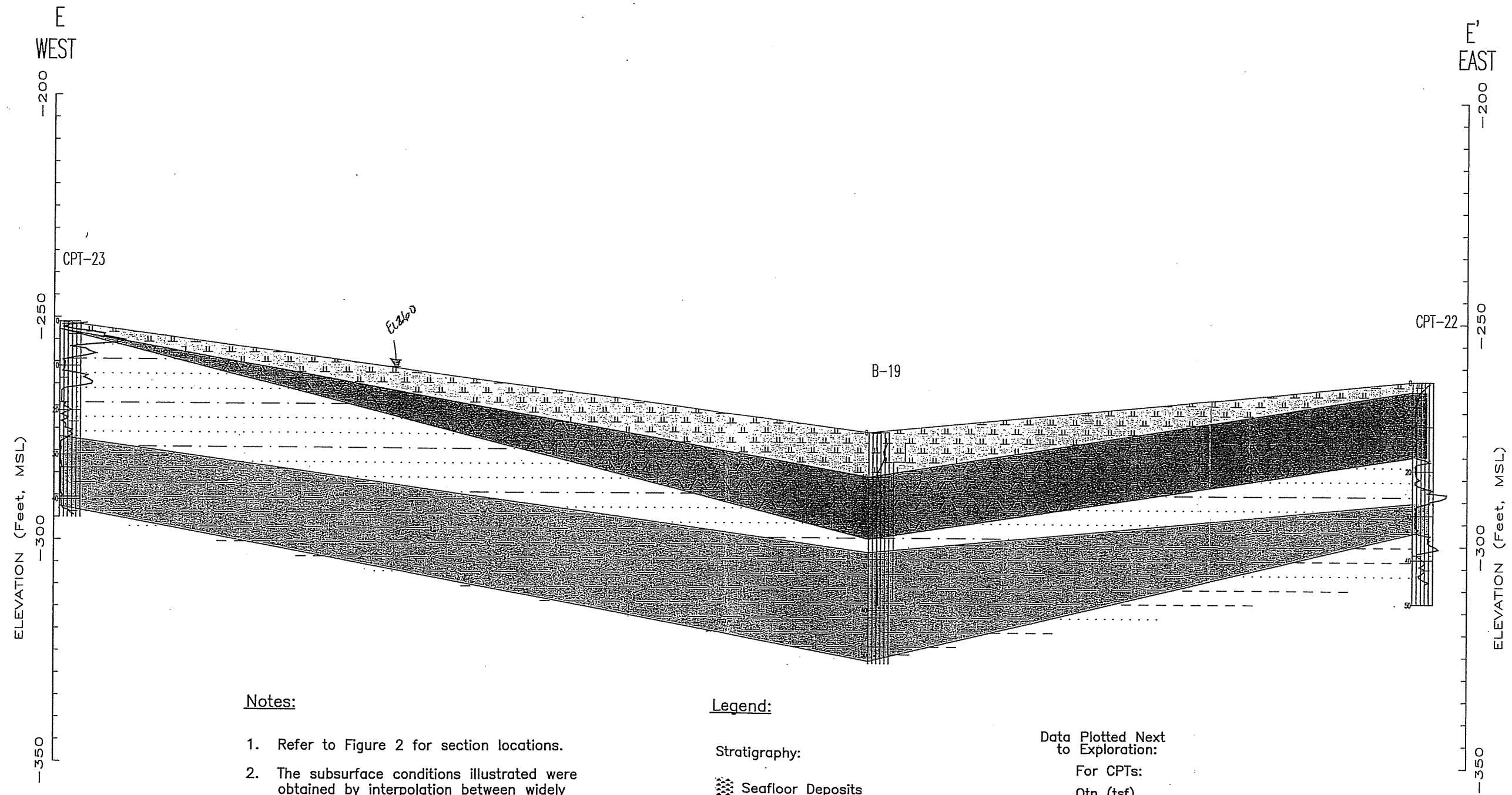
DATE: 1-29-04

PM: LDH

PROJ. NO: 27663042.00005

FIG. NO:

12



Notes:

1. Refer to Figure 2 for section locations.
2. The subsurface conditions illustrated were obtained by interpolation between widely spaced explorations. Information on actual subsurface conditions exists only at locations of explorations; conditions between explorations may vary from those depicted.
3. The discussions in the text of this report should be reviewed to obtain a proper understanding of the nature of the subsurface conditions.
4. Complete boring logs and CPT data are presented in the appendices of this report.

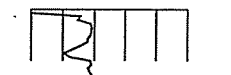
Legend:

- Stratigraphy:**
- Seafloor Deposits
 - Soft Lacustrine Deposits
 - Upper Alluvial Deposits
 - Upper Stiff Lacustrine Deposits
 - Lower Alluvial Deposits
 - Lower Stiff Lacustrine Deposits

Data Plotted Next to Exploration:

For CPTs:

Qtn (tsf)
0 50 100 150 200 250



For Borings:

W%
0 100



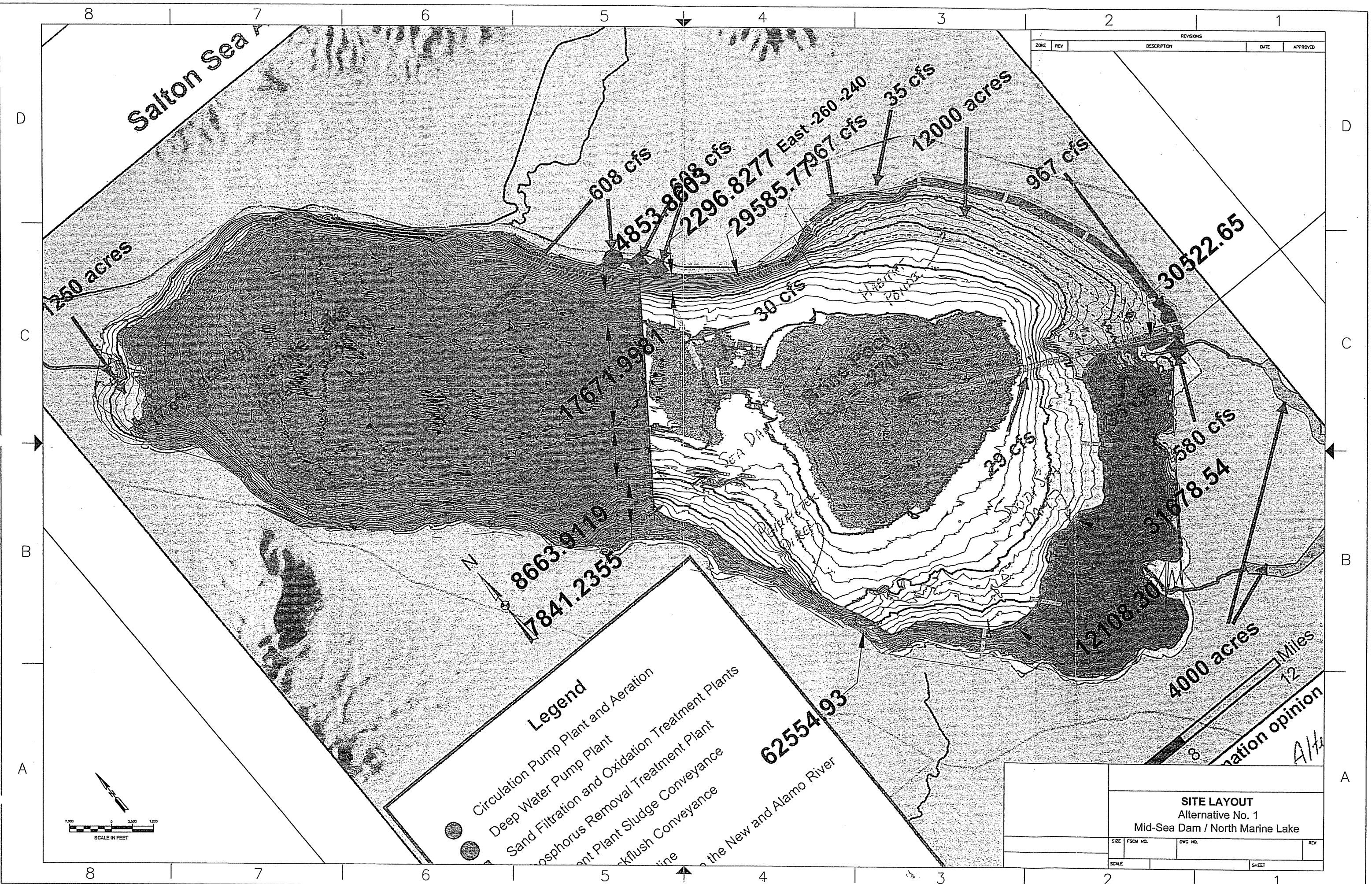
(VERTICAL SCALE = 200 X HORIZONTAL SCALE)

**GEOLOGIC CROSS SECTION E-E'
SALTON SEA RESTORATION PROJECT**

URS	2500 0 2500 5000 Feet SCALE: 1" = 5000'	CHECKED BY: AG	DATE: 1-29-04	FIG. NO: 13
		PM: LDH	PROJ. NO: 27663042.00005	

771100_01_0.dwg

C. London



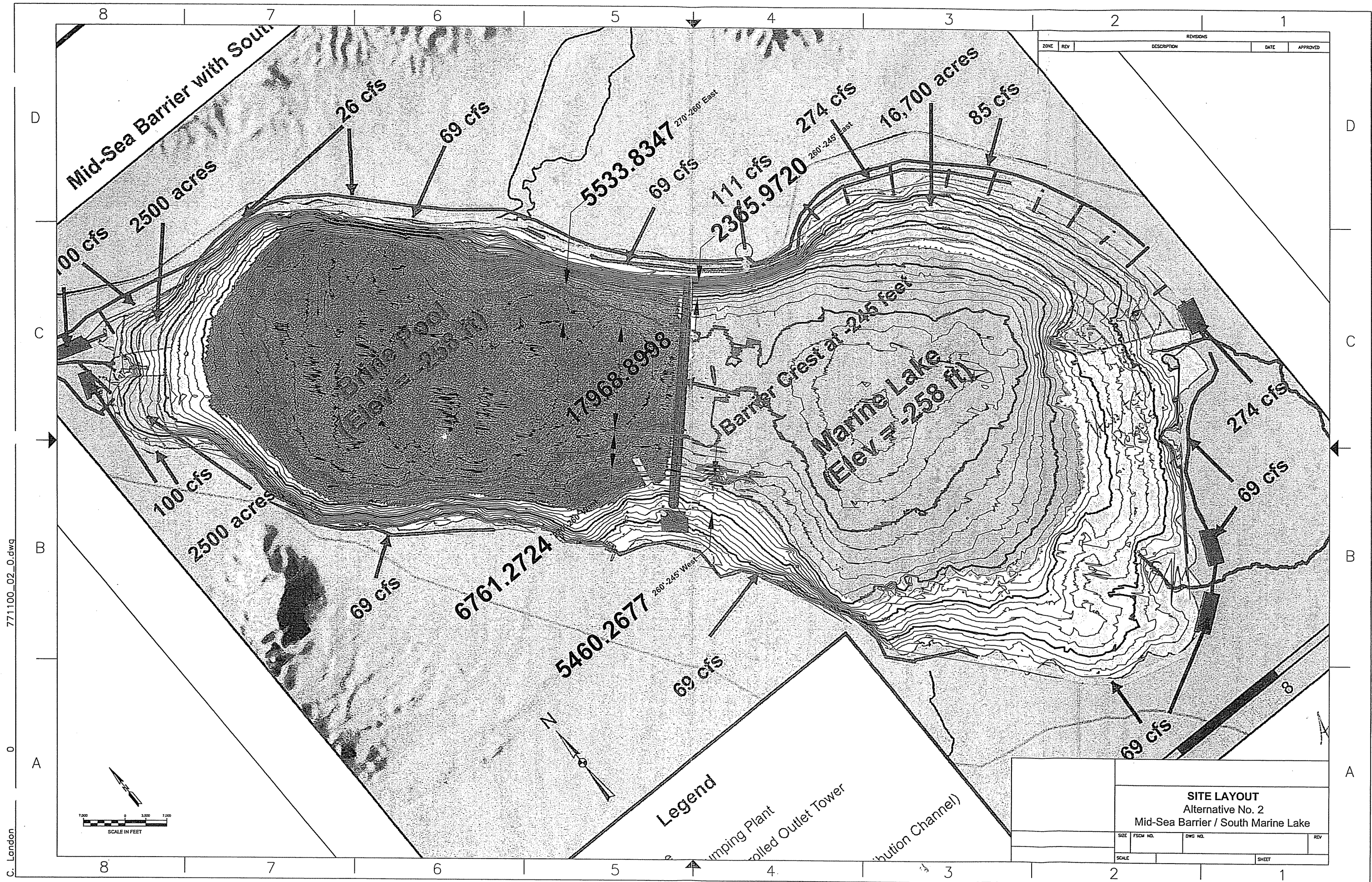
REVISIONS			
ZONE	REV	DESCRIPTION	DATE

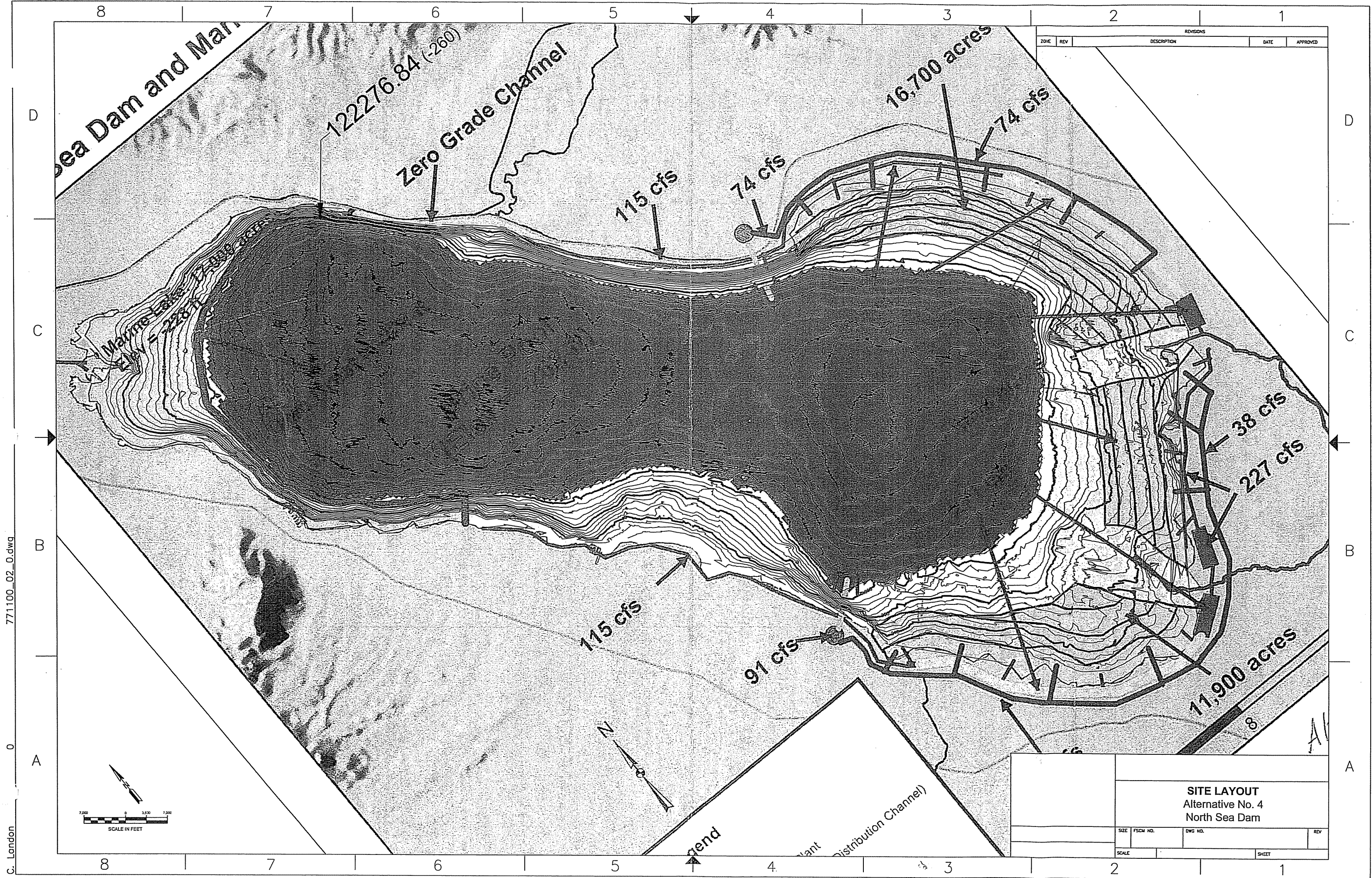
Legend

- Circulation Pump Plant and Aeration
- Deep Water Pump Plant
- Sand Filtration and Oxidation Treatment Plant
- Phosphorus Removal Treatment Plant
- Plant Sludge Conveyance
- Skidflush Conveyance

SITE LAYOUT
Alternative No. 1
Mid-Sea Dam / North Marine Lake

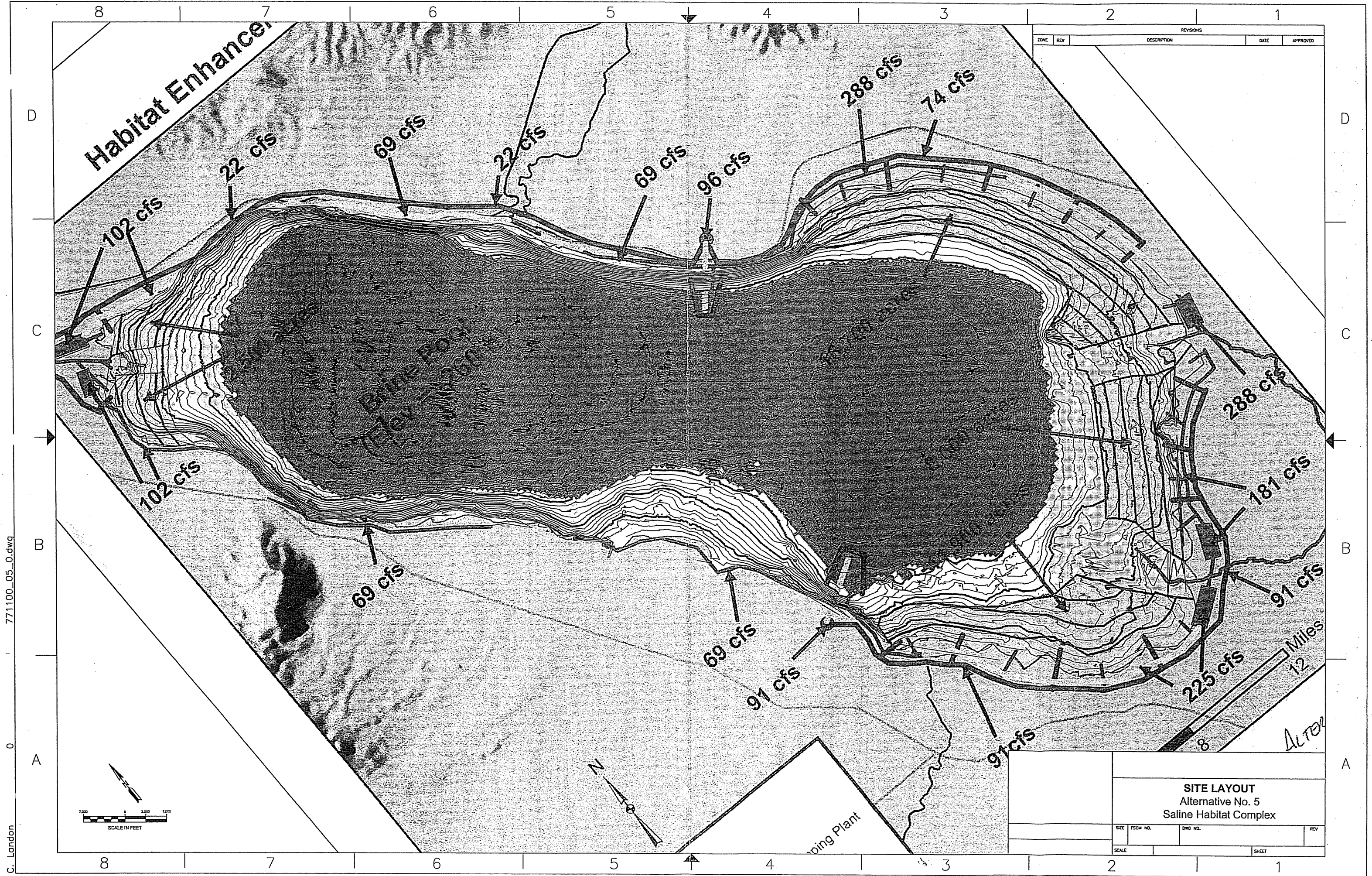
SCALE	SHEET





REVISIONS			
DATE	REV	DESCRIPTION	APPROVED

SITE LAYOUT Alternative No. 4 North Sea Dam			
SIZE	FSCH NO.	DWG NO.	REV
SCALE	SHEET		



REVISIONS			
ZONE	REV	DESCRIPTION	DATE

SITE LAYOUT Alternative No. 5 Saline Habitat Complex			
SIZE	FSOM NO.	DWG NO.	REV
SCALE	SHEET		1

771100_05_0.dwg
C. London

B. Embankment Length Estimation

Date _____ Weather _____ Hours _____ Miles _____ Sheet 2 of 1
Project SALTON SEA - HABITAT POND Submitted by JIE YU Date 8/25/06
Project Number 71100 Reviewed by CDS Date 8/25/06

EMBANKMENT DIKE LENGTH ESTIMATION:

FOR 8,600 ACRES OF HABITAT POND AREA (SEE ATTACHED FIGURE I:)
1" = 7000'

ESTIMATED LENGTH OF EMBANKMENT DIKE:

$$(3.2'' + 3.3'' + 3.5'' + 4.0'' + 4.7'' + 2.2'' + 2'') \cdot 7000 \text{ Ft/INCH}$$
$$= 22.9'' \cdot 7000 \text{ Ft/INCH} = 160300 \text{ FT}$$

$$\frac{160300 \text{ FT}}{8,600 \text{ ACRES}} = 18.64 \text{ FT/ACRE}$$

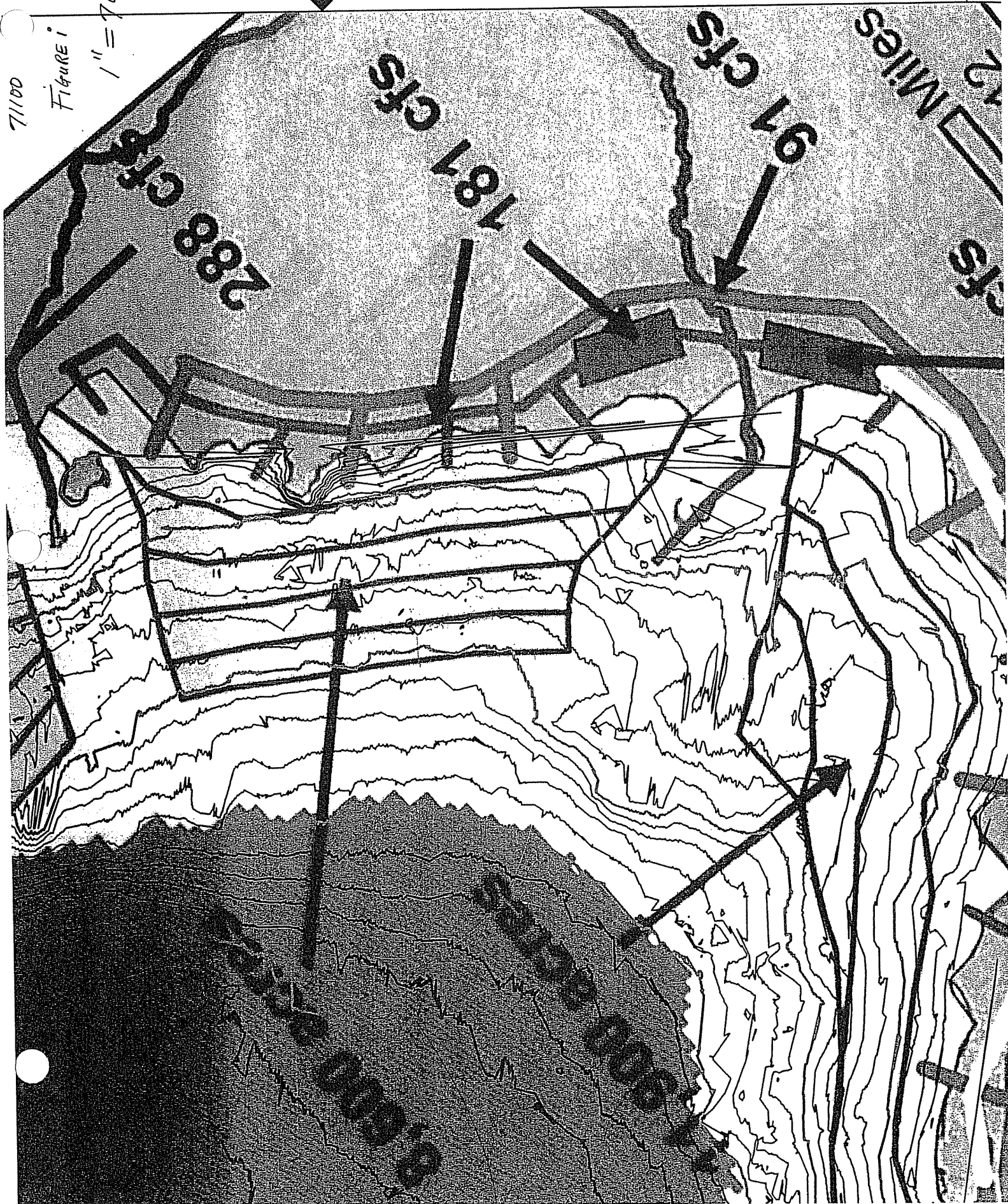
FOR A 2500 ACRES HABITAT POND AREA, ESTIMATED LENGTH OF
EMBANKMENT DIKE IS APPROXIMATELY 46600 FEET. ✓

B

1" = 7000'

Figure 1

7/1/00



C. Embankment Quantity Calculation



Date _____ Weather _____ Hours _____ Miles _____ Sheet 1 of 5
 Project SALTON SEA - MIDSEA DAM Submitted by JIE YU Date 8/21/06
 Project Number 71100 Reviewed by CDS Date 8/24/06

MIDSEA DAM CROSS SECTION CALCULATION:

1. ASSUMPTIONS

- ↓
1. CREST ELEVATION: $-225'$ AND CREST WIDTH: $30'$
 2. TOP OF FOUNDATION ELEVATION IS ASSUMED TO BE AT TOP OF SOFT LACUSTRINE AND/OR ALLUVIUM DEPOSITS
 3. BOTTOM OF FOUNDATION ELEVATION IS ASSUMED TO BE AT BOTTOM OF SOFT LACUSTRINE AND/OR ALLUVIUM DEPOSITS.

2. AREA CALCULATION.

SEE CROSS SECTION SCHEMATIC FIG. A

EMBANKMENT HEIGHT (FROM TOP OF CREST TO TOP OF FOUNDATION)

 H_E

FOUNDATION HEIGHT (FROM TOP TO BOTTOM OF FOUNDATION)

 H_F

i. SAND/GRAVEL EMBANKMENT FILL (TYPE A)

A. ABOVE FOUNDATION ELEVATION:

$$30 \cdot H_E + 3 \cdot H_E \cdot H_E = 3H_E^2 + 30H_E$$

B. BELOW FOUNDATION ELEVATION:

$$\{[(3 \cdot H_E) \cdot 2 + 30] + H_F\} \cdot H_F = [(6H_E + 30) + H_F] H_F = (6H_E + 30) \cdot H_F + H_F^2 \quad \checkmark$$

ii. TOTAL SAND/GRAVEL EMBANKMENT FILL (TYPE A)

$$\underline{3H_E^2 + 30H_E + (6H_E + 30) \cdot H_F + H_F^2} \quad \text{IN SQUARE FEET} \quad \checkmark$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 2 of 5
 Project SALTON SEA - MIDSEA DAM Submitted by JIE YU Date 8/21/06
 Project Number 71100 Reviewed by CDI Date 8/24/06

ii, SAND/GRAVEL SHELL

STONE COLUMN PLACEMENT PLATFORM TYPE A MATERIAL:

$$(3H_E) \cdot H_E + (1.5H_E) \cdot H_E = 4.5 H_E^2 \quad \text{IN SQ. FT.} \quad \checkmark$$

STONE COLUMN PLACEMENT PLATFORM TYPE B MATERIAL

$$(5H_E) \cdot H_E - (1.5H_E) \cdot H_E = 3.5 H_E^2 \quad \text{IN SQ. FT.} \quad \checkmark$$

iii LENGTH OF STONE COLUMN.

N_1 : Number of stone columns to be installed at upstream or downstream slope of the embankment at 10' interval per row.

$$N_1 = 3H_E/10 \text{ per slope} \quad \text{down} \quad \text{ROUND TO NEAREST INTEGER}$$

N_2 : Number of stone columns to be installed at crest of embankment ✓

$$N_2 = 2$$

LENGTH OF STONE COLUMNS FOR 8.66' OF EMBANKMENT (ONE ROW)

$$N_1 \cdot H_E + 2(N_1)H_F + N_2(H_E + H_F)$$

$$= (H_E + 2H_F) \cdot N_1 + 2 \cdot (H_E + H_F) \quad \checkmark$$

LENGTH OF STONE COLUMNS PER YARD OF EMBANKMENT

$$[(H_E + 2H_F) \cdot N_1 + 2(H_E + H_F)] \cdot 3/8.66 \quad \text{IN FEET} \quad \checkmark$$

$$N_1 = 18$$

$$(18H_E + 36H_F + 2H_E + 2H_F) \cdot 3/8.66 \quad \checkmark$$

$$= (20H_E + 38H_F) \cdot 3/8.66 \quad \checkmark$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 3 of 5
 Project SALTON SEA - MIDSEA DAM Submitted by JIE YH Date 8/21/06
 Project Number 71100 Reviewed by GDC Date 8/24/06

IV DREDGE OF SOFT LACUSTRINE DEPOSITS

$$[(3H_E) \cdot 2 + 30' + H_F] \cdot H_F = \underline{(6H_E + 30 + H_F) H_F} \quad \checkmark \quad (\text{IN SQUARE FEET})$$

V DREDGE OF SEAFLOOR DEPOSITS

ASSUME SEAFLOOR DEPOSITS HEIGHT: H_S

$$\{[(3H_E) \cdot 2 + 30 + (5H_E) \cdot 2] + 3H_S\} \cdot H_S \quad \checkmark$$

$$= \{[6H_E + 30 + 10H_E] + 3H_S\} \cdot H_S$$

$$= \underline{[16H_E + 3H_S + 30] \cdot H_S} \quad (\text{IN SQ FT}) \quad \checkmark$$

VI INSTALLATION OF SBC WALL WITH SYNTHETIC

ASSUMPTIONS: ① WIDTH OF SBC WALL SHOULD BE 5'

② BOTTOM OF SBC WALL SHOULD BE 40' BELOW
BOTTOM OF FOUNDATION

$$\therefore \text{LENGTH OF SBC WALL: } \underline{H_E + H_F + 40} \quad (\text{IN FEET}) \quad \checkmark \quad \checkmark$$

$$\therefore \text{AREA OF SBC WALL/PER YARD: } 3(H_E + H_F + 40) \quad (\text{IN SQ. FEET}) \quad \checkmark$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 4 of 5
 Project SALTON SEA - MID SEA DAM / NORTH SEA DAM Submitted by JTE YH Date 8/24/06
 Project Number 71100 Reviewed by CDS Date 8/25/06

vii 3' FILTER SAND BLANKET (TYPE A) OVER SOFT LACIOTRINE

$$5 H_E \cdot 3 = 15 H_E$$

IN SQ. FT.

viii 5' COARSE BLANKET DRAIN (TYPE B)

$$\text{BOTTOM LENGTH: } 3 H_E + 1.5 H_E + 15' - 50' - (1.5) 3' = 4.5 H_E - 39.5'$$

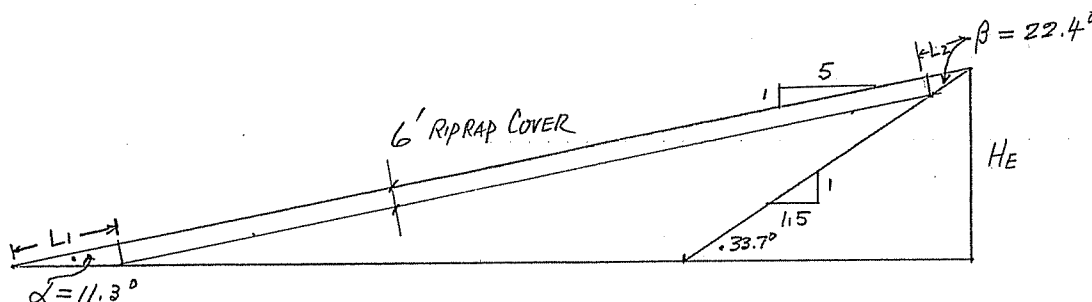
$$\text{HEIGHT: } 5'$$

$$\text{TOTAL AREA} = [(4.5 H_E - 39.5') - 5' (1.5)] \cdot 5' = (4.5 H_E - 47') \cdot 5' \text{ IN SQ. FT.}$$

$$\text{STONE COLUMN PORTION} = [3 H_E + 15 - 50 - 3(3)] \cdot 5 = [3 H_E - 44 - 11.3] \cdot 5 = 15 H_E - 276.5 \text{ IN SQ. FT.}$$

$$- \frac{5 \cdot (1.5) + 5(3)}{2}$$

ix RIPRAP SLOPE PROTECTION:



$$L_1 = \frac{6}{\tan 11.3^\circ} = 30'$$

$$L_1 + L_2 = 44.6'$$

$$L_2 = \frac{6}{\tan 22.4^\circ} = 14.6'$$

UPSTREAM OR DOWNSTREAM RIPRAP AREA

$$\left(\frac{H_E}{\sin 11.3^\circ} - L_1 - L_2 \right) \cdot 6 + \frac{1}{2} L_1 \cdot 6 + \frac{1}{2} L_2 \cdot 6$$

$$= \left(\frac{H_E}{\sin 11.3^\circ} - 44.6 \right) \cdot 6 + 3(L_1 + L_2) = \frac{6 H_E}{\sin 11.3^\circ} - 267.6 + 133.8 = \frac{6 H_E}{\sin 11.3^\circ} - 133.8$$

$$= 30.6 H_E - 133.8 \text{ IN SQ. FT.}$$



KLEINFELDER

DAILY REPORT

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Project Salton Sea Submitted by JIE YU Date 8/29/06
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X. Wick Drains.

$$2 \cdot \left(\frac{5 H_E}{5} + 1 \right) \cdot H_F = \frac{2 (H_E + 1) \cdot H_F}{\text{For 5' of Embankment.}}$$

in Ft



Date _____ Weather _____ Hours _____ Miles _____ Sheet 1 of 7
 Project Saltion Sea - MIDSEA BARRIER Submitted by JIE YU Date 9/14/2006
 Project Number 71100 Reviewed by ca Date 9/18/06

MIDSEA BARRIER (SEISMIC DESIGN) — SEE FIG 4.14

1. Assumption:

1. CREST ELEVATION: $-245'$; CREST WIDTH: $30'$
2. H_E : EMBANKMENT HEIGHT
3. H_F : FOUNDATION HEIGHT
4. H_S : SEAFLOOR DEPOSIT HEIGHT

2. AREA CALCULATION:

i, SAND/GRAVEL EMBANKMENT FILL (TYPE A)

A. ABOVE FOUNDATION:

$$30 \cdot H_E + 2 H_E \cdot H_E = 2 H_E^2 + 30 H_E \checkmark$$

B. BELOW FOUNDATION ELEVATION:

$$[(2 H_E) \cdot 2 + 30 - H_F] \cdot H_F$$

$$= (4 H_E + 30 - H_F) \cdot H_F \checkmark \quad \text{IN SQUARE FEET}$$

ii SAND/GRAVEL SHELL

STONE COLUMN PLACEMENT PLATFORM TYPE A MATERIAL

$$(1.5 H_E) \cdot H_E + (2.0 H_E) \cdot H_E = 3.5 H_E^2 \checkmark \quad \text{IN SQ FEET}$$

SAND/GRAVEL SHELL TYPE B MATERIAL

$$(3.0 H_E) \cdot H_E - (1.5 H_E) \cdot H_E = 1.5 H_E^2 \checkmark \quad \text{IN SQ. FEET.}$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 2 of 7
 Project SALTON SEA - MID SEA BARRIER Submitted by JIE YU Date 9/14/2006
 Project Number 71100 Reviewed by ca Date 9/18/06

iii LENGTH OF STONE COLUMN:

N_1 : Number of Stone Columns to be installed at upstream or downstream
 slope of the Embankment at 10' interval per row

$$N_1 = 2.0 H_E / 10 \quad \text{IF } H_E = 41'; \quad \frac{2.0 H_E}{10'} = \frac{2 \cdot 41'}{10'} = 8.2$$

use $N_1 = 8$ ✓

N_2 : Number of Stone Columns to be installed at crest of Embankment

$$N_2 = 2 \quad \checkmark$$

LENGTH OF STONE COLUMNS FOR 8.66' OF Embankment (ONE ROW)

$$\begin{aligned}
 & N_1 \cdot H_E + 2(N_1) \cdot H_F + N_2(H_E + H_F) \\
 &= (H_E + 2H_F) \cdot N_1 + 2(H_E + H_F) \quad \checkmark
 \end{aligned}$$

iv DREDGE OF SOFT LACUSTRINE DEPOSITS

$$[(2H_E) \cdot 2 + 30 - H_F] \cdot H_F = (4H_E + 30 - H_F) \cdot H_F \quad \checkmark \quad \text{IN SQ FT.}$$

v DREDGE OF SEAFLOOR DEPOSITS:

$$\begin{aligned}
 & [(2H_E) \cdot 2 + 30 + (3H_E) \cdot 2 + 3H_s] \cdot H_s \\
 &= [4H_E + 30 + 6H_E + 3H_s] \cdot H_s \\
 &= (10H_E + 3H_s + 30) \cdot H_s \quad \checkmark
 \end{aligned}$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 3 of 7
 Project SALTION SEA - MID SEA BARRIER Submitted by JIE YU Date 9/14/2006
 Project Number _____ Reviewed by CO Date 9/18/06

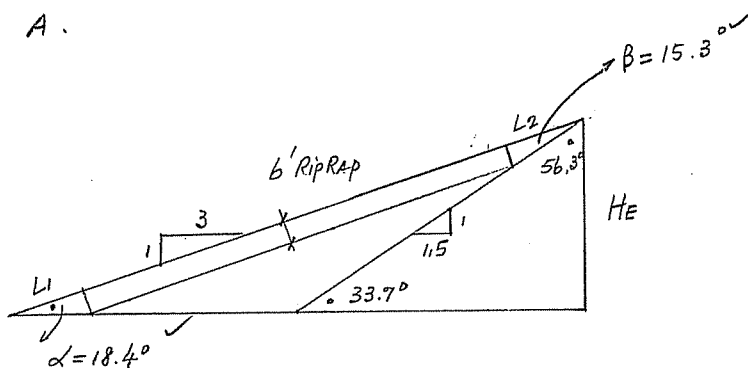
VII INSTALLATION OF SCB WALL

1. WIDTH OF SCB WALL SHOULD BE 5'
2. BOTTOM OF SCB WALL SHOULD BE 40' BELOW BOTTOM OF FOUNDATION

∴ LENGTH OF SCB WALL : $H_E + H_F + 40$ ✓ IN FT

VII RIPRAP SLOPE PROTECTION

A.



$$L_1 = \frac{6}{\tan 18.4^\circ} = 18.0'$$

$$\therefore L_1 + L_2 = 39.9'$$

$$L_2 = \frac{6}{\tan 15.3^\circ} = 21.9'$$

UPSTREAM OR DOWNSTREAM RIPRAP AREA

$$\begin{aligned} & \left(\frac{H_E}{\sin 18.4^\circ} - L_1 - L_2 \right) \cdot 6 + \frac{1}{2} L_1 \cdot 6 + \frac{1}{2} L_2 \cdot 6 \\ &= \left(\frac{H_E}{\sin 18.4^\circ} - 39.9' \right) \cdot 6 + 3(39.9') = \frac{6}{\sin 18.4^\circ} \cdot H_E - 3(39.9') \\ &= 19H_E - 119.7 \quad \text{IN SQ FT} \end{aligned}$$

→ B. RIPRAP ON TOP OF CREST

$$\begin{aligned} & [(2.0H_E) \cdot 2 + 30 + 6 \cdot 1.5] \cdot 6 \\ &= (4H_E + 30 + 9) \cdot 6 \\ &= (4H_E + 39) \cdot 6 = 24H_E + 234 \quad \text{IN SQ FT} \end{aligned}$$

**KLEINFELDER****DAILY REPORT**

Date _____ Weather _____ Hours _____ Miles _____ Sheet 4 of 7
Project SALTON SEA - MIDSEA BARRIER Submitted by JIE YU Date 9/14/06
Project Number _____ Reviewed by CS Date 9/18/06

viii Wick DRAINS

Assume : Wick DRAINS will be installed at 5' x 5' GRID IN
FOUNDATION SOIL

$$\left[\left(\frac{3 H_E}{5} + 1 \right) \cdot H_F \right] \cdot 2$$
$$= 2 H_F \left(\frac{3}{5} H_E + 1 \right) \checkmark$$



Date _____ Weather _____ Hours _____ Miles _____ Sheet 5 of 7
 Project SALTON SEA - MIDSEA BARRIER Submitted by JIE YU Date 9/14/06
 Project Number 71100 Reviewed by CA Date 9/13/06

MIDSEA BARRIER (NON-SEISMIC DESIGN) — SEE FIG 4.14

1. ASSUMPTIONS:

1. CREST ELEVATION: $-245'$; CREST WIDTH: $30'$

2. H_E : EMBANKMENT HEIGHT

3. H_F : FOUNDATION HEIGHT

4. H_S : SEAFLOOR DEPOSITS HEIGHT

2. AREA CALCULATION:

i, SAND/GRAVEL EMBANKMENT FILL (TYPE A)

A. ABOVE FOUNDATION

$$30 H_E + 1.5 H_E \cdot H_E = 1.5 H_E^2 + 30 H_E \quad \checkmark$$

IN SQ FT

B. BELOW FOUNDATION

$$[(1.5 H_E) \cdot 2 + 30 - H_F] \cdot H_F$$

$$= (3 H_E + 30 - H_F) \cdot H_F \quad \checkmark$$

IN SQ FT

ii, SAND/GRAVEL SHELL (TYPE B)

$$(5 H_E) \cdot H_E - (1.5 H_E) \cdot H_E = 3.5 H_E^2 \quad \checkmark$$

IN SQ FT.

iii LENGTH OF STONE COLUMNS

(NOT APPLICABLE) $\checkmark$

iv DREDGE OF SOFT LACUSTRINE DEPOSITS

$$[(1.5 H_E) \cdot 2 + 30 - H_F] \cdot H_F = (3 H_E + 30 - H_F) \cdot H_F \quad \checkmark$$

IN SQ FT

Date _____ Weather _____ Hours _____ Miles _____ Sheet 6 of 7
 Project SALTON SEA - MIDSEA BARRIER Submitted by JIE YU Date 9/14/06
 Project Number 71100 Reviewed by COL Date 9/18/06

V. DREDGE OF SEAFLOOR DEPOSITS

$$[(5H_E) \cdot 2 + 30 + 3H_s] \cdot H_s$$

$$= (10H_E + 30 + 3H_s) \cdot H_s \quad \checkmark \quad \text{IN SQ FT}$$

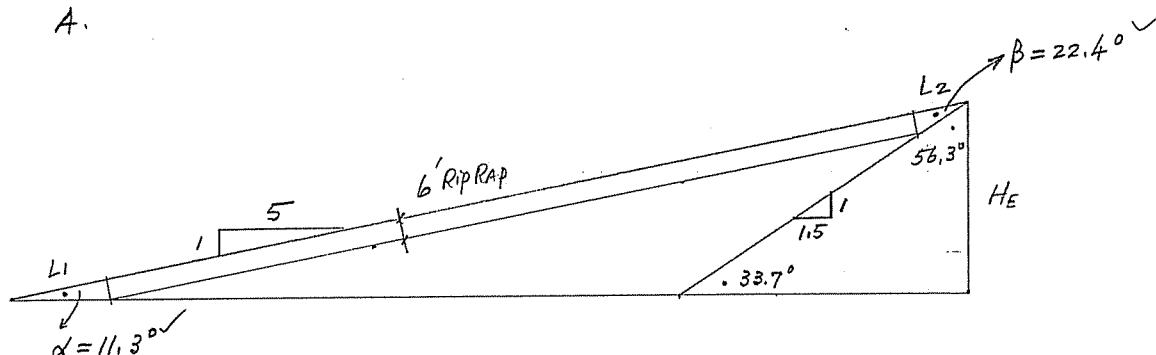
VI. INSTALLATION OF SCB WALL

Assume: 1. WIDTH OF SCB WALL SHOULD BE 5'
 2. BOTTOM OF SCB WALL SHOULD BE 40' below
 BOTTOM OF FOUNDATION

$$\therefore \text{LENGTH OF SCB WALL: } H_E + H_F + 40 \quad \checkmark \quad \text{IN FT}$$

VII. RIPRAP SLOPE PROTECTION:

A.



$$L_1 = \frac{6}{\tan 11.3^\circ} = 30'$$

$$\therefore L_1 + L_2 = 44.6'$$

$$L_2 = \frac{6}{\tan 22.4^\circ} = 14.6'$$

upstream OR Downstream Riprap AREA

$$\left(\frac{H_E}{\sin 11.3^\circ} - L_1 - L_2 \right) \cdot 6 + \frac{1}{2} L_1 \cdot 6 + \frac{1}{2} L_2 \cdot 6 = \frac{6 H_E}{\sin 11.3^\circ} - 3(L_1 + L_2)$$

$$= 30.6 H_E - 133.8 \quad \checkmark \quad \text{IN SQ FT}$$

**KLEINFELDER****DAILY REPORT**

Date _____ Weather _____ Hours _____ Miles _____ Sheet 7 of 7
Project SALTON SEA - MIDSEA BARRIER Submitted by JIE YU Date 9/14/06
Project Number 71100 Reviewed by LS Date 9/18/06

B. RIPRAP ON TOP OF CREST

$$(30 + 6 \cdot 1.5) \cdot 6 = 39 \cdot 6 = 234 \checkmark$$

IN SQ FT.

VIII Wick DRAINS

Assume: Wick DRAINS will be installed at 5' x 5' GRID IN Foundation Soil.

$$\begin{aligned} & \left[\left(\frac{5H_E - 1.5H_E}{5} + 1 \right) \cdot H_F \right] \cdot 2 \\ & = 2H_F \left(\frac{3.5H_E}{5} + 1 \right) \checkmark \end{aligned}$$



Date _____ Weather _____ Hours _____ Miles _____ Sheet 1 of 4
 Project SOUTH SEA DAM - FAULT MOVEMENT ^{RESISTANCE VERSION} Submitted by JIE YU Date 9/29/2006
 Project Number 71100 Reviewed by _____ Date _____

South Sea Dam:

1. Assumption:

1. Crest Elevation: -220, crest width: 30'
2. H_E : Embankment Height
3. H_F : Foundation Height
4. H_S : Seafloor Deposits Thickness
5. upstream pool El: -230

2. AREA Calculation:

i, SAND/GRAVEL Embankment Fill (TYPE A)

A. Embankment

$$30 \cdot H_E + (1.5 H_E) \cdot H_E = 1.5 H_E^2 + 30 H_E \quad \text{IN SQ FT}$$

B. FOUNDATION

$$[2(1.5 H_E) + 30 - H_F] \cdot H_F = (3 H_E + 30 - H_F) \cdot H_F \quad \text{IN SQ FT}$$

ii SAND/GRAVEL SHELL

STONE COLUMN PLACEMENT PLATFORM TYPE A MATERIAL

$$(1.5 H_E) \cdot H_E + (1.5 H_E) \cdot H_E = 3 H_E^2 \quad \text{IN SQ FT}$$

SAND/GRAVEL SHELL TYPE B MATERIAL

$$2 \cdot (20') \cdot H_E = 40 H_E \quad \text{IN SQ FT}$$



Date _____ Weather _____ Hours _____ Miles _____ Sheet 2 of 4
 Project South Sea dam - Fault resistance Submitted by JIE YU Date 9/29/2006
 Project Number 7/100 Reviewed by _____ Date _____

iii LENGTH OF STONE Column:

N_1 : NUMBER OF STONE ^{Columns} TO BE INSTALLED AT upstream OR DOWN stream slope OF Embankment

$$N_1 = \frac{1.5 H_E}{10} \quad \text{Round down TO NEAREST INTERGER}$$

N_2 : NUMBER OF STONE Columns TO BE INSTALLED AT crest OF Embankment

$$N_2 = 2$$

LENGTH OF STONE Column FOR 8.66' OF Embankment (one row)

$$N_1 \cdot H_E + 2(N_1) \cdot H_F + N_2(H_E + H_F) \\ = (H_E + 2H_F) \cdot N_1 + 2(H_E + H_F)$$

iv DREDGE OF SOFT LACUSTRINE DEPOSITS

$$[(1.5H_E) \cdot 2 + 30 - H_F] \cdot H_F = (3H_E + 30 - H_F) \cdot H_F \quad \text{IN SQ FT}$$

v DREDGE OF SEAFLOOR DEPOSITS

$$[(1.5H_E) \cdot 2 + 30 + (1.5H_E) \cdot 2 + (40') \cdot 2 + (2.5H_E - 1.5H_E) \cdot 2 + 3H_S] \cdot H_S \\ = [3H_E + 30 + 3H_E + 80 + 2H_E + 3H_S] \cdot H_S \\ = [8H_E + 110 + 3H_S] \cdot H_S$$

vi INSTALLATION OF SCB WALL

Assume: WIDTH OF SCB WALL IS 9'

BOTTOM OF SCB WALL SHOULD BE 40' BELOW BOTTOM OF FOUNDATION

$$\therefore \text{LENGTH OF SCB WALL} = H_E + H_F + 40$$

IN FT

Date _____ Weather _____ Hours _____ Miles _____ Sheet 3 of 4
 Project South Sea Dam - Fault resistance Submitted by JIE YU Date 9/29/2006
 Project Number 71100 Reviewed by _____ Date _____

VI 5' COARSE BLANKET DRAIN LAYER

$$\text{Bottom LENGTH: } 1.5H_E + 1.5H_E + 15' - 20' = 3H_E - 5'$$

$$\text{Height: } 5'$$

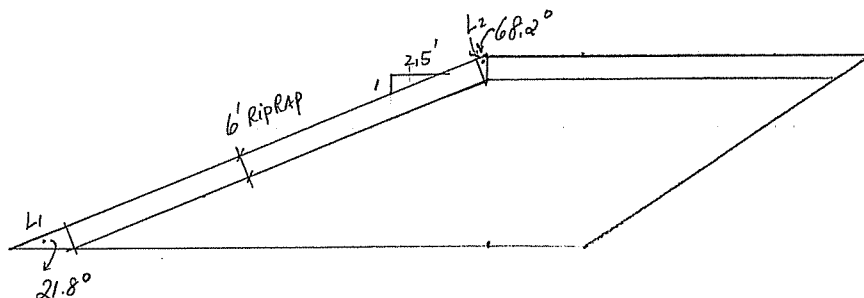
$$\text{TOTAL AREA} = [3H_E - 5' - 5'(1.5)] \cdot 5'$$

$$= (3H_E - 5' - 7.5') \cdot 5' = 15H_E - 62.5 \quad \text{IN SQ FT}$$

STONE Column portion:

$$(15H_E - 62.5) - (1.5H_E) \cdot 5 = 7.5H_E - 62.5 \quad \text{IN SQ FT}$$

VII RipRap



$$L_1 = \frac{6}{\tan 21.8^\circ} = 15'$$

$$L_2 = \frac{6}{\tan 68.2^\circ} = 2.4'$$

$$L_1 + L_2 = 15 + 2.4 = 17.4'$$

→ A. upstream or downstream RipRap AREA

ONE
SIDE
OF
EMBANK-
MENT

$$\left(\frac{H_E}{\sin 21.8^\circ} - L_1 - L_2 \right) \cdot 6 + \frac{1}{2} L_1 \cdot 6' + \frac{1}{2} L_2 \cdot 6' = \left(\frac{H_E}{\sin 21.8^\circ} - 17.4' \right) \cdot 6 + 3(17.4')$$

$$= \frac{6H_E}{\sin 21.8^\circ} - 3(17.4') = 16.2H_E - 52.2 \quad \text{IN SQ FT}$$

→ B. RipRap ON TOP OF FINE Rock Fill

$$\left[20' - \frac{1}{2} \cdot 6 \cdot (1.5) \right] \cdot 6' = (20 - 4.5) \cdot 6 = 93 \quad \text{IN SQ FT}$$

$$A + B = 16.2H_E - 52.2 + 93 = 16.2H_E + 40.8 \quad \text{IN SQ FT}$$



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 Project South Sea Dam - Fault resistance Submitted by JIE YU Date 9/29/2006
 Project Number 71100 Reviewed by _____ Date _____

viii Wick Drain

Assume: Wick Drain will be installed at a 5' x 5' Grid

$$2 \cdot \left[\frac{1.5 H_E + 40' + (2.5 H_E - 1.5 H_E)}{5} + 1 \right] \cdot H_F$$

$$= 2 \cdot \left[\frac{2.5 H_E + 40'}{5} + 1 \right] \cdot H_F = 2 H_F \left(\frac{2.5 H_E + 40'}{5} + 1 \right)$$

ix FINE Rock Fill:

$$2 \cdot \left\{ [(2.5 H_E - 1.5 H_E) + 20'] + 20' \right\} \frac{H_E}{2} - \text{RIP RAP AREA}$$

$$= \{ (H_E + 20') + 20' \} \cdot H_E - 2(16.2 H_E + 40.8)$$

$$= (H_E + 40') \cdot H_E - 32.4 H_E - 81.6$$

$$= \underline{H_E^2 + 7.6 H_E - 81.6}$$

IN SQ FT

Date _____ Weather _____ Hours _____ Miles _____ Sheet 1 of 5
 Project Salton SEA - perimeter Dike and South Sea Dam Submitted by JIE YU Date 8/23/2006
 Project Number 71100 Reviewed by CDS Date 8/24/06

PERIMETER DIKE AND SOUTH SEA DAM:

1. Assumption:

1. Crest Elevation: -225; Crest Width: 20' ✓
2. H_E : Embankment Height
3. H_F : Foundation Height
4. H_S : Seafloor Deposit Height
5. upstream pool El: -230

2. AREA CALCULATION:

i) SAND/GRAVEL Embankment Fill (Type A)

A. Embankment

$$20 \cdot H_E + (1.5 H_E) \cdot H_E = 1.5 H_E^2 + 20 H_E \quad \checkmark \quad \text{IN SQ. FT.} \quad \checkmark$$

B. Foundation

$$[2(1.5 H_E) + 20' - H_F] \cdot H_F = (3 H_E + 20 - H_F) \cdot H_F \quad \checkmark \quad \text{IN SQ. FT.} \quad \checkmark$$

ii) SAND/GRAVEL SHELL

Stone Column placement platform Type A Material:

$$(1.5 H_E) \cdot H_E + (1.5 H_E) \cdot H_E = 3 H_E^2 \quad \checkmark \quad \text{IN SQ. FT.} \quad \checkmark$$

Stone Column placement platform Type B Material:

$$(2.5 H_E) \cdot H_E - (1.5 H_E) \cdot H_E = 1 H_E^2 \quad \checkmark \quad \text{IN SQ. FT.} \quad \checkmark$$

iii) Length of Stone Column

N_1 : Number of Stone Columns to be installed at upstream

OR Downstream slope of the Embankment at 10' interval per row

perimeter dike: $N_1 = \frac{1.5 H_E}{10} = \frac{1.5 \cdot 18.3}{10} = 2.75$ use $N_1 = 2$ for perimeter dike ✓
 South Sea dam: $N_1 = \frac{1.5 H_E}{10} = \frac{1.5 \cdot 27}{10} = 4.05$ use $N_1 = 4$ for South Sea Dam. ✓

N_2 : Number of Stone Column to be installed at CREST OF Embankment

$$N_2 = 2 \quad \checkmark$$



KLEINFELDER

DAILY REPORT

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 Project Salton Sea - perimeter Dike and South Sea dam Submitted by JIE YU Date 8/23/2006
 Project Number 71100 Reviewed by ODS Date 8/24/06

LENGTH OF STONE Column FOR 8.66' OF Embankment (ONE ROW)

$$N_1 \cdot H_E + 2(N_1) \cdot H_F + N_2(H_E + H_F) = (H_E + 2H_F)N_1 + 2LH_E + H_F$$

LENGTH OF STONE Column FOR PER YARD OF Embankment

$$(1.5H_E + 8H_F) \cdot 3 / 8.66 \quad \text{in FT.}$$

LD in Perimeter Dike

iv DREDGE OF SOFT LAMSTRINE Deposits

$$[(1.5H_E) \cdot 2 + 20 - H_F] \cdot H_F = (3H_E + 20 - H_F) \cdot H_F \quad \text{in SQ. FT.}$$

v DREDGE OF SEAFLOOR Deposits

$$[(1.5H_E) \cdot 2 + 20 + (2.5H_E) \cdot 2 + 3H_S] \cdot H_S$$

$$= [3H_E + 20 + 5H_E + 3H_S] \cdot H_S$$

$$= (8H_E + 20 + 3H_S) \cdot H_S$$

IN SQ. FT.

vi' INSTALLATION OF SBC WALL WITH SYNTHETIC

Assume: 1. WIDTH OF SBC WALL should be 3'

2. Bottom of SBC WALL should be 40' below
Bottom of Foundation

$$\therefore \text{length of SBC WALL: } H_E + H_F + 40 \quad \text{in FT}$$

\therefore AREA OF SBC WALL / PER YARD OF Embankment:

$$3(H_E + H_F + 40) \quad \text{in SQ FT}$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 3 of 5
 Project Salton Sea - perimeter Dike and South Sea dam Submitted by JIE YU Date 8/25/06
 Project Number 71100 Reviewed by CDS Date 8/25/06

VI 3' COARSE BLANK DRAIN LAYER (TYPE C)

For perimeter dike Bottom length: $1.5H_E + 1.5H_E + 10 - 10 = 3H_E$ ✓

HEIGHT: 3'

TOTAL AREA = $[3H_E - 3'(1.5)] \cdot 3' = (3H_E - 4.5) \cdot 3 = 9H_E - 13.5$ ✓ in SQ. FT

STONE Column portion = $(9H_E - 13.5) - (1.5H_E) \cdot 3 = 4.5H_E - 13.5$ in SQ. FT.

For South Sea DAM

Bottom length: $1.5H_E + 1.5H_E + 10 - 20 = 3H_E - 10$

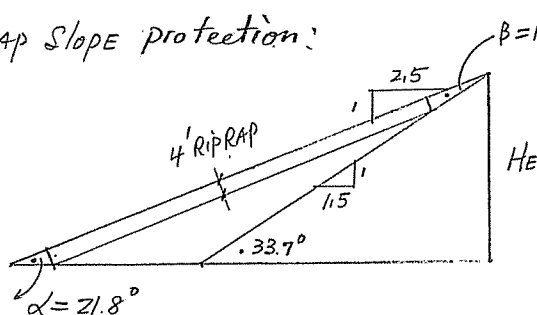
HEIGHT: 3'

TOTAL AREA = $[3H_E - 10' - 3'(1.5)] \cdot 3' = (3H_E - 14.5) \cdot 3 = 9H_E - 43.5$ ✓ SQ. FT

STONE Column portion = $(9H_E - 43.5) - (1.5H_E) \cdot 3 = 4.5H_E - 43.5$ in SQ. FT. ✓

VII Riprap Slope protection:

For perimeter Dike:



$$L_1 = \frac{4}{\tan 21.8^\circ} = 10$$

$$L_1 + L_2 = 29$$

$$L_2 = \frac{4}{\tan 11.9^\circ} = 19$$

A. upstream OR DownStream Riprap AREA

$$\left(\frac{H_E}{\sin 21.8} - L_1 - L_2 \right) \cdot 4 + \frac{1}{2} L_1 \cdot 4 + \frac{1}{2} L_2 \cdot 4 = \left(\frac{H_E}{\sin 21.8} - 29 \right) 4 + 2 \cdot (29)$$

$$= 10.8 H_E - 58 \quad \text{in SQ. FT} \quad \checkmark$$

B. Riprap ON Top OF CREST

$$[(1.5H_E) \cdot 2 + 20 + 4 \cdot 1.5] \cdot 4 = (3H_E + 26) \cdot 4 = 12H_E + 104 \quad \text{in SQ. FT.} \quad \checkmark$$

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 Project Saltton SEA - perimeter Dike and South Sea dam Submitted by JIE YU Date 8/25/2006
 Project Number 71100 Reviewed by QDS Date 8/25/06

For South Sea Dam:

See Above Figure: use 6' of RipRap

$$L_1 = \frac{6}{\tan 21.8^\circ} = 15 \checkmark$$

$$L_1 + L_2 = 43.5$$

$$L_2 = \frac{6}{\tan 11.9^\circ} = 28.5 \checkmark$$

A. upstream OR Downstream RipRap AREA

$$\begin{aligned} & \left(\frac{H_E}{\sin 21.8^\circ} - L_1 - L_2 \right) \cdot 6 + \frac{1}{2} L_1 \cdot 6 + \frac{1}{2} L_2 \cdot 6 \\ &= \left(\frac{H_E}{\sin 21.8^\circ} - 43.5 \right) \cdot 6 + 3 \cdot 43.5 = 16.2 H_E - 3 \cdot 43.5 \\ &= \underline{16.2 H_E - 130.5} \quad \checkmark \text{ IN SQ. FT} \end{aligned}$$

B. RIPRAP ON TOP OF CREST

$$\begin{aligned} & [(1.5 H_E) \cdot 2 + 20 + 6 \cdot 1.5] \cdot 6 \\ &= (3 H_E + 29) \cdot 6 = \underline{18 H_E + 174} \quad \checkmark \text{ IN SQ. FT.} \end{aligned}$$

**KLEINFELDER****DAILY REPORT**Date _____ Weather _____ Hours _____ Miles _____ Sheet 5 of 5Project Salton SEA - perimeter Dike and South Sea Dam Submitted by JIE YU Date 8/25/2006Project Number _____ Reviewed by CDI Date 8/25/06

VIII Wick DRAINS

Assume: Wick Drains will be installed at a 5'x5' grid
in Foundation

For perimeter dike:

$$2\left(\frac{2.5H_E}{5} + 1\right) \cdot H_F = \frac{2\left(\frac{H_E}{2} + 1\right) \cdot H_F}{\text{For 5' of Embankment}} \quad \checkmark \quad \text{IN FT}$$

For South Sea Dam:

$$2\left(\frac{2.5H_E}{5} + 1\right) \cdot H_F = \frac{2\left(\frac{H_E}{2} + 1\right) \cdot H_F}{\text{For 5' of Embankment}} \quad \checkmark \quad \text{IN FT}$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 1 of 3
Project SALTON SEA - NORTH SEA DAM Submitted by JIE YU Date 8/24/2006
Project Number 71100 Reviewed by CDS Date 8/24/06

NORTH SEA DAM

1. ASSUMPTION:

1. CREST ELEVATION: -223, Crest WIDTH: 30'
2. H_E : Embankment HEIGHT
3. H_F : Foundation HEIGHT
4. H_S : SEA FLOOR DEPOSITS HEIGHT

2. AREA CALCULATION:

i. SAND/GRAVEL EMBANKMENT FILL (TYPE A)

A. ABOVE FOUNDATION ELEVATION:

$$(30) \cdot H_E + (3H_E) \cdot H_E = 3H_E^2 + 30H_E \quad \checkmark \quad \text{IN SQ. FT.}$$

B. BELOW FOUNDATION ELEVATION:

$$\begin{aligned} \{ [(3 \cdot H_E) \cdot 2 + 30] + H_F \} \cdot H_F &= (6H_E + 30 + H_F) \cdot H_F \\ &= (6H_E + 30) \cdot H_F + H_F^2 \quad \checkmark \quad \text{IN SQ. FT.} \end{aligned}$$

ii. SAND/GRAVEL SHELL

Stone Column placement platform TYPE A MATERIAL:

$$(3H_E) \cdot H_E + (1.5H_E) \cdot H_E = 4.5H_E^2 \quad \checkmark \quad \text{IN SQ. FT.}$$

Stone Column placement platform TYPE B material:

$$(5H_E) \cdot H_E - (1.5H_E) \cdot H_E = 3.5H_E^2 \quad \checkmark \quad \text{IN SQ. FT.}$$



Date _____ Weather _____ Hours _____ Miles _____ Sheet 2 of 3
 Project SALTON SEA - NORTH SEA DAM Submitted by JIE YU Date 8/24/2006
 Project Number 71100 Reviewed by QDS Date 8/24/06

iii LENGTH OF STONE COLUMN

N_1 : Number of Stone Columns to be installed at upstream or down stream slope of the embankment at 10' interval per row down

$$N_1 = \frac{3 \cdot H_E}{10} \quad \text{ROUND TO NEAREST INTEGER}$$

N_2 : Number of Stone columns to be installed at crest of embankment

$$N_2 = 2$$

LENGTH OF STONE Columns FOR 8.66' OF Embankment (ONE Row)

$$N_1 \cdot H_E + 2(N_1) \cdot H_F + N_2(H_E + H_F)$$

$$= (H_E + 2H_F) \cdot N_1 + 2(H_E + H_F) \quad \checkmark$$

iv DREDGE OF SOFT LACUSTRINE DEPOSITS

$$[(3H_E) \cdot 2 + 30 + H_F] \cdot H_F = (6H_E + 30 + H_F) \cdot H_F \quad \checkmark \quad \text{IN SQ. FT.}$$

v DREDGE OF SEA FLOOR DEPOSITS

$$\{[(3H_E) \cdot 2 + 30 + (5H_E) \cdot 2] + 3H_S\} \cdot H_S$$

$$= \{[6H_E + 30 + 10H_E] + 3H_S\} \cdot H_S$$

$$= (16H_E + 30 + 3H_S) \cdot H_S \quad \checkmark \quad \text{IN SQ. FT.}$$

vi INSTALLATION OF SBC WALL WITH SYNTHETIC

- ASSUMPTIONS:
1. WIDTH OF SBC WALL should be 5'
 2. Bottom of SBC WALL should be 40' Below Bottom of Foundation

$$\therefore \text{length of SBC WALL: } H_E + H_F + 40 \quad (\text{IN FEET})$$



KLEINFELDER

DAILY REPORT

Date _____ Weather _____ Hours _____ Miles _____ Sheet 3 of 3
 Project SALTON SEA - MID SEA DAM / NORTH SEA DAM Submitted by JTE YH Date _____
 Project Number 71100 Reviewed by CDS Date 8/25/06

VII 3' FILTER SAND BLANKET (TYPE A) OVER SOFT LAGOTRINE

$$5 H_E : 3 = 15 H_E$$

IN SQ. FT.

VIII 5' COARSE BLANKET DRAIN (TYPE B)

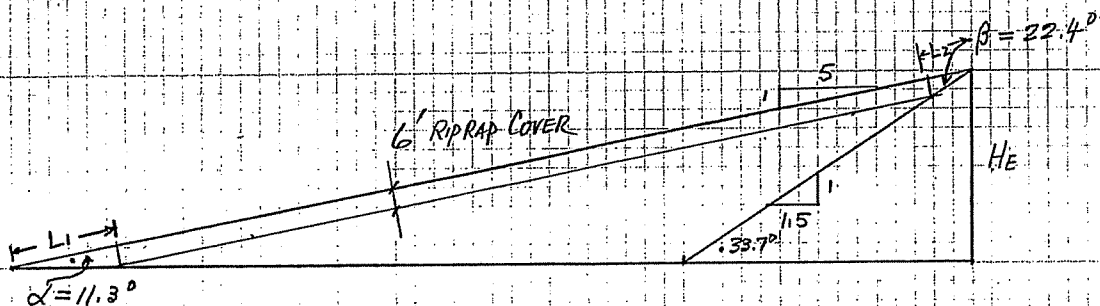
$$\text{BOTTOM LENGTH: } 3 H_E + 1.5 H_E + 15' - 50' - (1.5) 3' = 4.5 H_E - 39.5'$$

$$\text{HEIGHT: } 5'$$

$$\text{TOTAL AREA} = [(4.5 H_E - 39.5') - 5' (1.5)] \cdot 5' = (4.5 H_E - 47') \cdot 5' \text{ IN SQ. FT.}$$

$$\text{STONE COLUMN PORTION} = [3 H_E + 15 - 50 - \frac{5 \cdot (1.5) + 5(3)}{2}] \cdot 5 = [3 H_E - 44 - 11.3] \cdot 5 = 15 H_E - 276.5 \text{ IN SQ. FT.}$$

IX RIPRAP SLOPE PROTECTION:



$$L_1 = \frac{6}{\tan 11.3^\circ} = 30'$$

$$L_1 + L_2 = 44.6'$$

$$L_2 = \frac{6}{\tan 22.4^\circ} = 14.6'$$

UPSTREAM OR DOWNSTREAM RIPRAP AREA

$$\left(\frac{H_E}{\sin 11.3^\circ} - L_1 - L_2 \right) \cdot 6 + \frac{1}{2} L_1 \cdot 6 + \frac{1}{2} L_2 \cdot 6$$

$$= \left(\frac{H_E}{\sin 11.3^\circ} - 44.6 \right) \cdot 6 + 3 (L_1 + L_2) = \frac{6 H_E}{\sin 11.3^\circ} - 267.6 + 133.8 = \frac{6 H_E}{\sin 11.3^\circ} - 133.8$$

IN SQ. FT.

NOTE: Wick DRAIN: SEE MID SEA DAM.

Date 10/27/2006 Weather _____ Hours _____ Miles _____ Sheet 1 of 2
 Project SALTON SEA - CONCENTRIC RING DIKE Submitted by JIE YU Date 10/27/2006
Without Stone Columns
 Project Number 71100 Reviewed by _____ Date _____

CONCENTRIC RING DIKE Without Stone Columns. — SEE LOWER PORTION OF FIG 5.3

1. Assumption:

1. CREST ELEVATION = 10' ABOVE SEAFLOOR; CREST WIDTH = 20'
2. H_E : EMBANKMENT HEIGHT
3. H_F : FOUNDATION HEIGHT
4. H_S : SEAFLOOR DEPOSITS HEIGHT

2. AREA CALCULATION

i. SAND/GRAVEL EMBANKMENT FILL (TYPE A)

A. EMBANKMENT

$$20 \cdot H_E + (1.5 H_E) \cdot H_E = \underline{1.5 H_E^2 + 20 H_E} \quad \text{IN SQ. SF}$$

B. FOUNDATION

$$[2(1.5 H_E) + 20 - H_F] \cdot H_F = \underline{(3 H_E + 20 - H_F) \cdot H_F} \quad \text{IN SQ. SF}$$

ii. SAND/GRAVEL EMBANKMENT FILL (TYPE B)

$$2 \cdot \left[(4 H_E - 1.5 H_E) \cdot H_E \cdot \frac{1}{2} \right] = (2.5 H_E) H_E = \underline{2.5 H_E^2} \quad \text{IN SQ. SF}$$

iii. DREDGE OF SOFT Lacustrine Deposits

$$[(1.5 H_E) \cdot 2 + 20 - H_F] \cdot H_F = \underline{(3 H_E + 20 - H_F) \cdot H_F} \quad \text{IN SQ. SF}$$

iv. DREDGE OF SEAFLOOR DEPOSITS

$$[(4 H_E) \cdot 2 + 20 + 3 H_S] \cdot H_S = \underline{[8 H_E + 20 + 3 H_S] \cdot H_S}$$

Date 10/27/2006 Weather _____ Hours _____ Miles _____ Sheet _____ of _____
 Project SALTON SEA - CONCENTRIC RING DIKE Submitted by JIE YU Date 10/27/06
WITHOUT STONE COLUMNS
 Project Number 71100 Reviewed by _____ Date _____

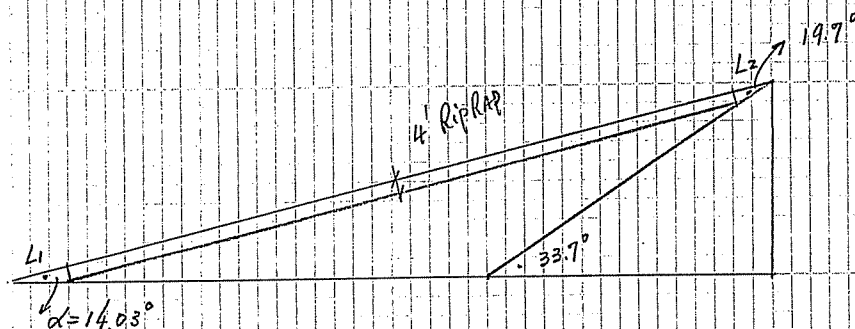
V. 3' coarse Blank Drain layer (Type B)

Bottom length: $1.5H_E + 10 - 10 = 1.5H_E$ IN SQ FT

HEIGHT: 3'

TOTAL AREA: $[1.5H_E - 3 \cdot 1.15] \cdot 3' = 4.5H_E - 13.5$ IN SQ FT

VI' Riprap:



$$L_1 = \frac{4}{\tan 14.0^\circ} = 16.04'$$

$$L_2 = \frac{4}{\tan 19.7^\circ} = 11.12'$$

$$L_1 + L_2 = 27.6'$$

A. upstream or Downstream Riprap AREA

$$\left(\frac{H_E}{\sin 14.0^\circ} - L_1 - L_2 \right) \cdot 4 + \frac{1}{2} L_1 \cdot 4 + \frac{1}{2} L_2 \cdot 4$$

$$= \left(\frac{H_E}{\sin 14.0^\circ} - 27.6 \right) \cdot 4 + 2(27.6) = \frac{4H_E}{\sin 14.0^\circ} - 55.2$$

$$= 16.5H_E - 55.2 \text{ IN SQ FT.}$$

B. Riprap ON TOP OF CREST

$$[20 + 4 \cdot 1.15] \cdot 4 = 104 \text{ IN SQ FT.}$$

VII Wick DRAINS:

$$2 \left[(4H_E - 1.5H_E) / 5 + 1 \right] \cdot H_F = \left(\frac{H_E}{2} + 1 \right) \cdot 2 \cdot H_F$$

Date _____ Weather _____ Hours _____ Miles _____ Sheet 2 of 4
 Project SALTON SEA — HABITAT POND Submitted by JIE YU Date 8/25/2006
 Project Number 71100 Reviewed by CDJ Date 8/25/06

HABITAT POND:

1. Assumption:

- 1, HEIGHT OF EMBANKMENT = $H_E = 7.5'$
2. HEIGHT OF FOUNDATION = $H_F = 10'$
- 3, CREST WIDTH = $15'$
4. FOUNDATION WIDTH AT BOTTOM OF EXCAVATION = $40'$

(SEE FIGURE 5.4)

2. AREA CALCULATION

i, FOUNDATION EXCAVATION AND EMBANKMENT FILL

$$[(2 \cdot H_F) + 40] \cdot H_F = [2 \cdot 10 + 40] \cdot 10 = \underline{600 \text{ SQ. FT.}} \quad \checkmark$$

ii FILTER/DRAIN BLANKET

$$\text{Bottom LENGTH: } \frac{15'}{2} + 3 \cdot H_E - 5' = 3H_E + 2.5'$$

$$\text{HEIGHT: } 2'$$

$$\text{AREA: } \left[(3H_E + 2.5') - \frac{1}{2}(2 \cdot 1.5) - \frac{1}{2}(2 \cdot 3) \right] \cdot 2$$

$$= (3H_E + 2.5' - 1.5 - 3) \cdot 2$$

$$= (3H_E - 2) \cdot 2 = 20.5 \cdot 2 = \underline{41 \text{ SQ. FT.}} \quad \checkmark$$

iii DIKE EMBANKMENT FILL

$$(15 + 3 \cdot H_E) \cdot H_E - 41 = (15 + 3 \cdot 7.5) \cdot 7.5 - 41$$

$$= (15 + 22.5) \cdot 7.5 - 41 = 281.3 - 41 = \underline{240.3 \text{ SQ. FT.}} \quad \checkmark$$

iv AREA OF GEOGRID/GEOTEXTILE PER FOOT OF EMBANKMENT

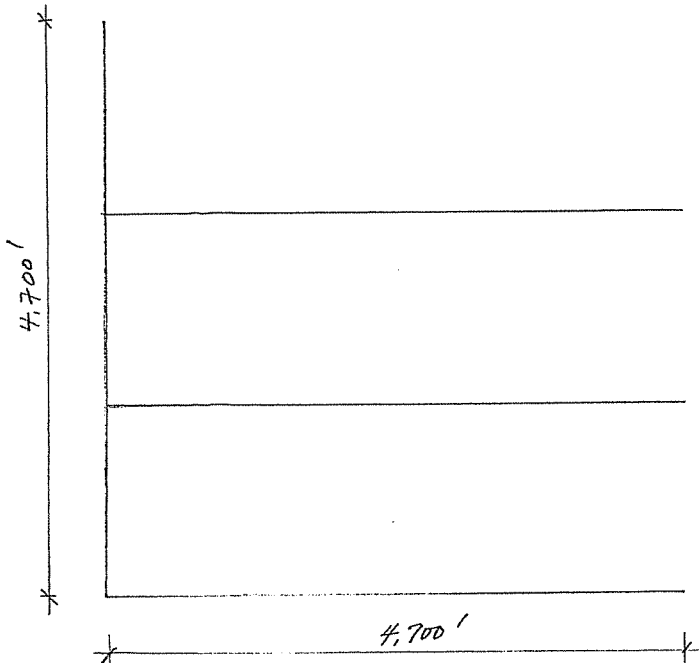
$$(40 + 2 \cdot \sqrt{10^2 + 20^2}) \cdot 1 = \underline{84.7 \text{ SQ FT}} \quad \checkmark$$



Date _____ Weather _____ Hours _____ Miles _____ Sheet 3 of 4
 Project SALTON SEA - HABITAT POND Submitted by JIE YU Date 12/12/2006
 Project Number 71100 Reviewed by RAA Date 4/13/07

3. 500-ACRE HABITAT POND EMBANKMENT LENGTH ESTIMATION:

WE ASSUME THAT A 500 ACRE HABITAT POND WILL INCLUDE FOUR(4) EMBANKMENT SECTIONS. THE EMBANKMENT LENGTH FOR EACH SECTION IS APPROXIMATELY 4,700 FEET.



THE ASSUMED LAYOUT OF THE FOUR(4) EMBANKMENT SECTIONS WITHIN A 500-ACRE HABITAT POND UNIT IS PRESENT LEFT.

∴ THE TOTAL EMBANKMENT LENGTH:

$$(4) \cdot (4,700') = 18,800 \text{ FEET}$$

FOR 500-ACRE HABITAT POND UNIT.

4. EXCAVATION AND EMBANKMENT QUANTITY CALCULATION:

i. EXCAVATION:

$$\frac{18,800'}{3} \cdot \frac{600 \text{ SF}}{9} = 417,778 \text{ C.Y.}$$

ii. EMBANKMENT:

$$\text{DIKE: } \frac{18,800'}{3} \cdot \frac{240.3}{9} = 167,320 \quad (167320)$$

FILTER MATERIAL:

$$\frac{18,800'}{3} \cdot \frac{41.}{9} = 28,548 \quad (28548)$$

FOUNDATION:

$$\frac{18,800'}{3} \cdot \frac{600}{9} = 417,778 \quad (417778)$$

$$\therefore \text{TOTAL EMBANKMENT INCLUDING FILTER MATERIAL: } 167,320 + 28,548 + 417,778 = 613,646 \text{ C.Y.}$$

(613,646 C.Y.)

**KLEINFELDER****DAILY REPORT**

Date _____ Weather _____ Hours _____ Miles _____ Sheet 4 of 4
Project SALTON SEA - HABITAT POND Submitted by JIE YU Date 12/12/2006
Project Number 71100 Reviewed by RAA Date 4/13/07

5. GEOGRID / GEOTEXTILE QUANTITY CALCULATION:

$$(18,800' \cdot 84.7 \frac{SF}{FOOT}) / 9 = 176,929 \text{ S.Y.} \checkmark$$

6. AERATE :

$$613,646 \text{ C.Y.} / 22,680 \text{ C.Y. / WEEK} = 27 \text{ WEEKS.} \checkmark$$

↳ PROVIDED BY MR. MIKE PANETTO

PROJECT Salton SEA RESTORATION
SUBJECT QUANTITY ESTIMATION NOTESPROJECT NO. 71100

BY _____ DATE _____

REVIEWED BY _____ DATE _____

MID SEA BARRIER

sand & gravel type A in core = subtract the top layer of
rip rap

sand and gravel type A in shell = subtract the total length of rip rap
over the top of embankment minus the layer of the core

sand and gravel type B = subtract rip rap over side slopes

MID SEA DAM + NORTH SEA

sand and gravel type A in core = subtract the section of coarse
filter within the core

sand and gravel type A in shell = subtract the sand and coarse
filter that will be in the shell

sand & gravel type B in shell = subtract the total rip rap and
subtract the section of sand filter that is in
the type B shell

Perimeter Dike + South Sea Dam

sand & gravel type A in core = subtract the rip rap over the top of the
and the section of coarse filter from the core

sand & gravel type A shell = subtract the rip rap along the top minus
the section over the core and subtract the total coarse
grained filter minus the section in the core

sand & gravel type B = subtract rip rap along the sides