

Seismic Hazard Basics

Best Practices in Dam and Levee Risk Analysis

Part B Hazard and Loading

Chapter B-2

Last modified July 2018, presented July 2019



US Army Corps
of Engineers®



Outline / Key Concepts

Introduction

- Seismic Stability Analyses in a Risk-informed Framework

Deterministic Seismic Hazard Analysis (DSHA)

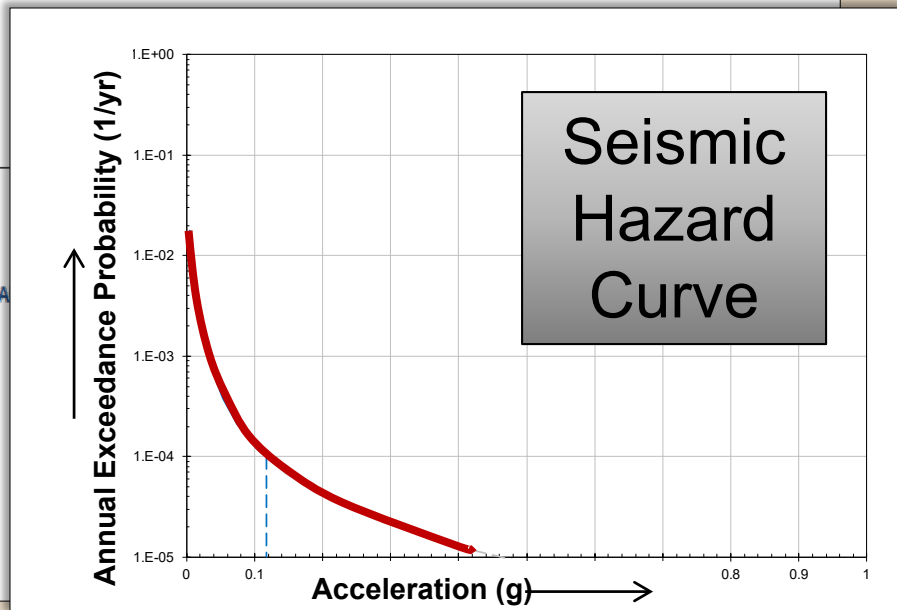
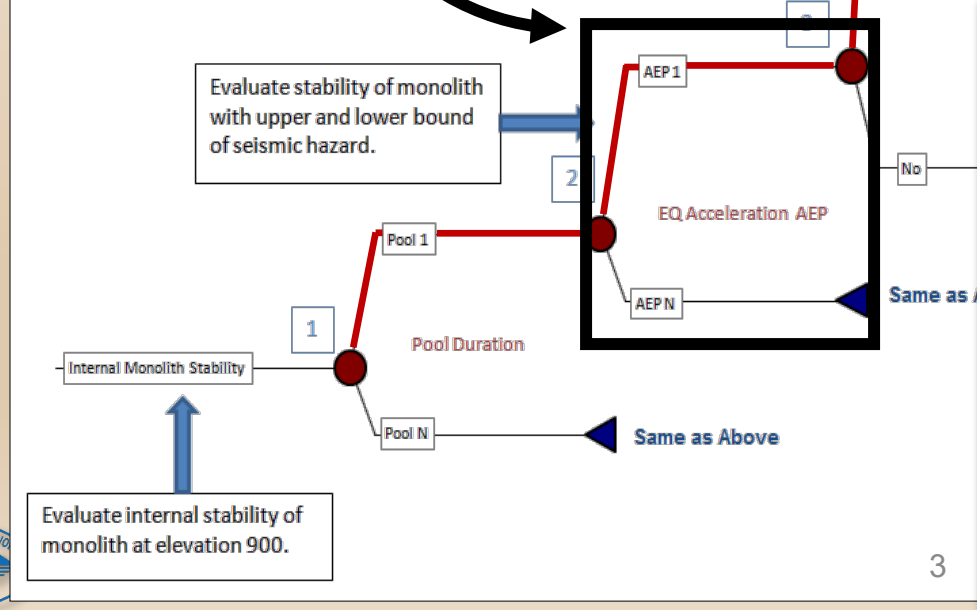
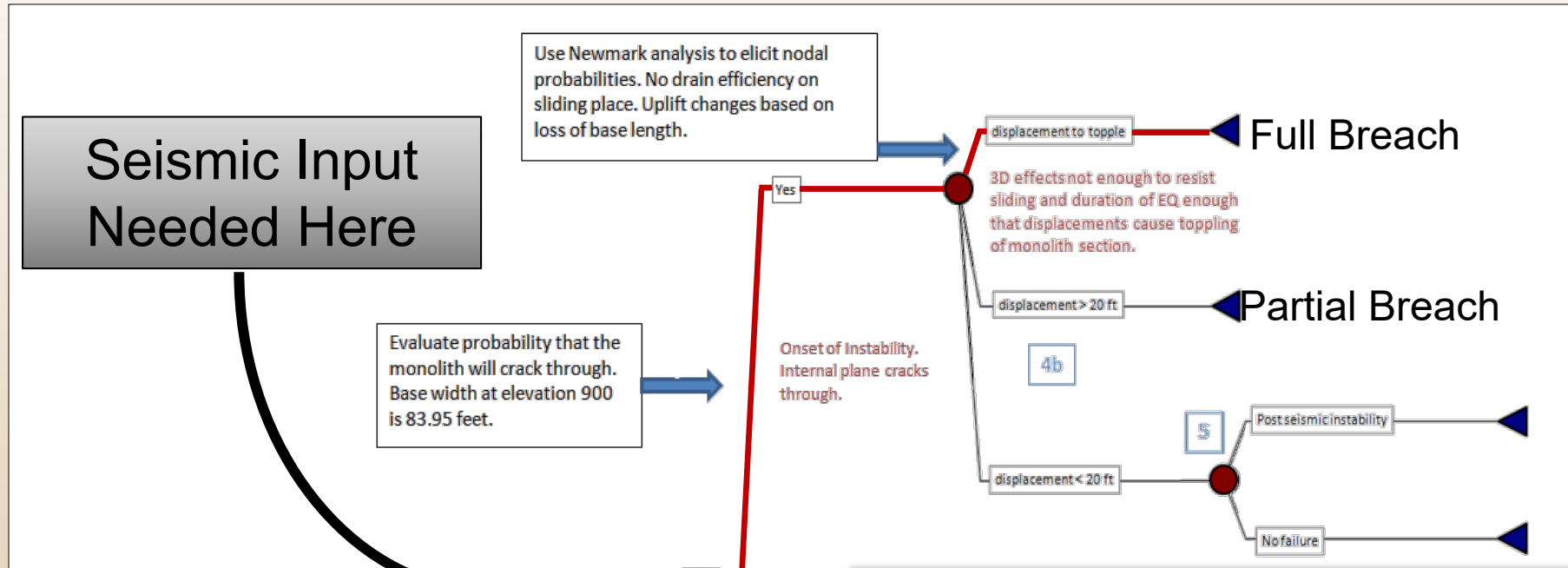
- Ground Motion Analyses: Basic Components
- “Worst-Case” vs. “Reasonable”

Probabilistic Seismic Hazard Analysis (PSHA)

- Seismic Source Characterization (Areal Sources, Fault Sources)
- Estimating Expected Strong Ground Motions
- Developing Seismic Hazard Curves
- Source-specific Contributions to Hazard
- Ground Motion Time Histories



Seismic Loading in Risk Assessments: PFMA Event Tree



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Seismic Hazard Analyses

Deterministic (DSHA):

- Selects a few scenarios (magnitude, distance, standard dev. [“epsilon”])
- For dams, typically “worst-case” **earthquake** (Max Credible EQ, “MCE”)
- Chooses largest **expected ground motion** from selected scenarios

Probabilistic (PSHA):

- Considers all scenarios (magnitude, distance, epsilon)
- Computes the rate of every scenario
- Combines rates of all scenarios and selected ground motion thresholds to evaluate probabilities of exceedance of strong shaking

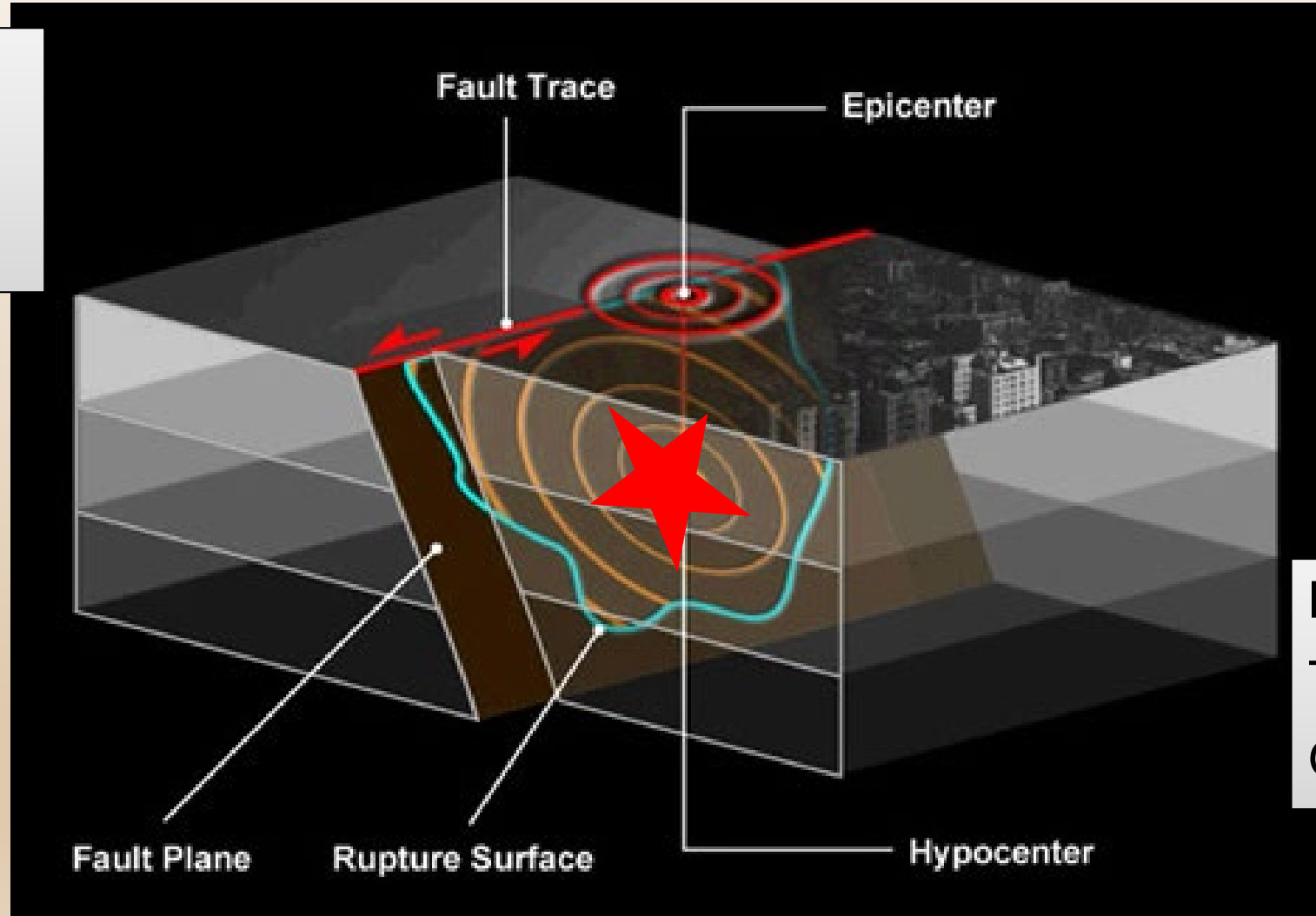


Ground Motion Analysis: Basic Components

Source Effects

Path Effects

Site Effects



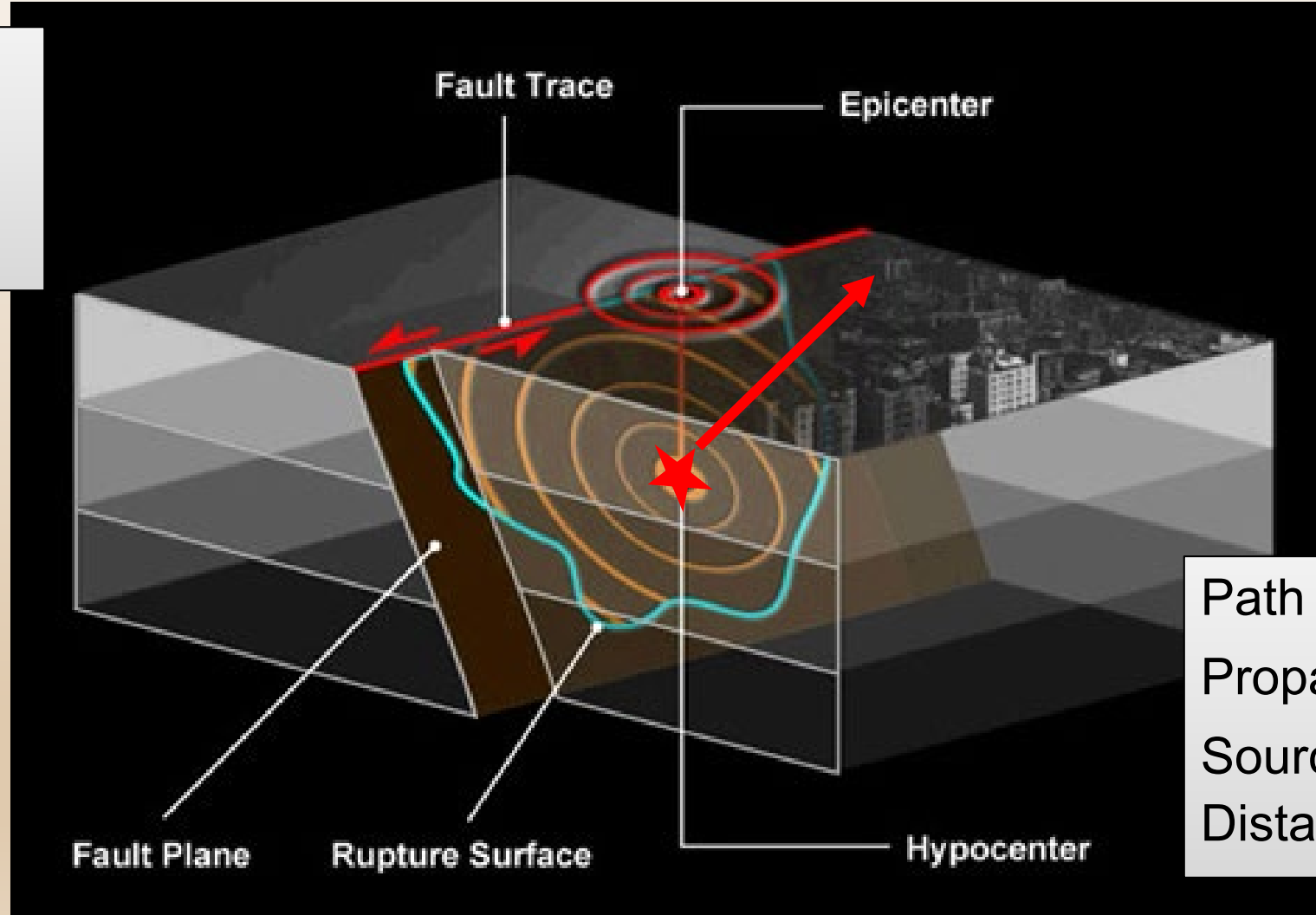
Magnitude
Type of Slip
Occurrence Rate

Ground Motion Analysis: Basic Components

Source Effects

Path Effects

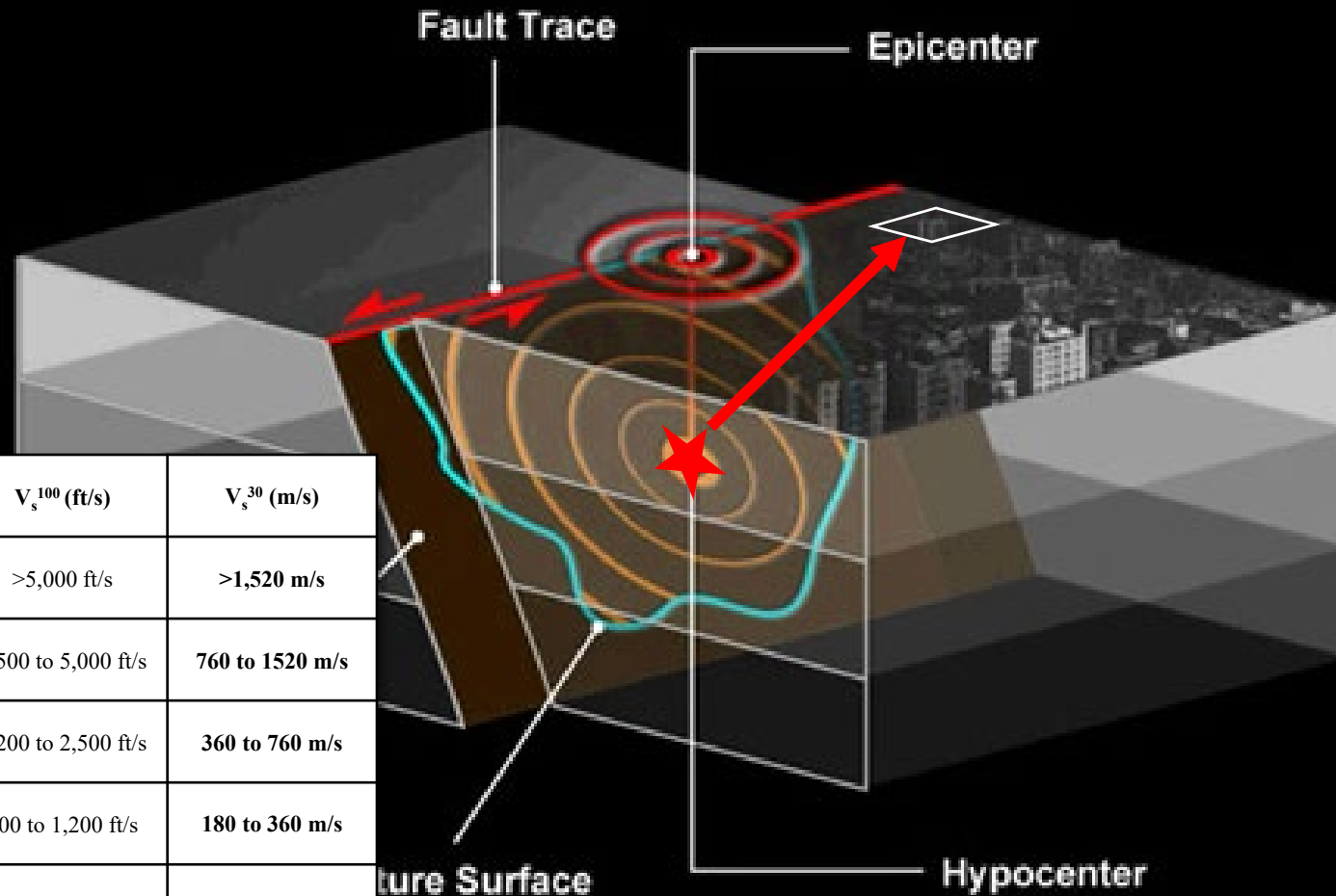
Site Effects



Path Geology
Propagation Direction
Source-to-Site
Distance

Ground Motion Analysis: Basic Components

Source Effects
Path Effects
Site Effects



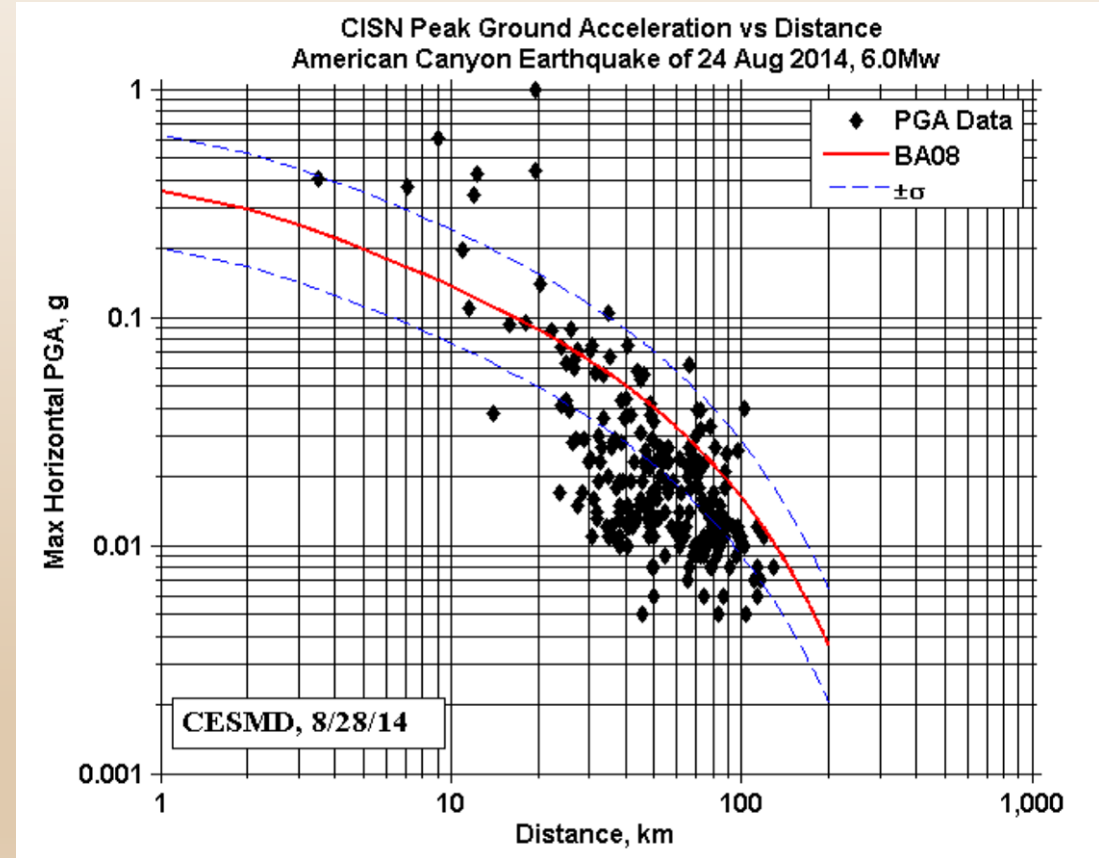
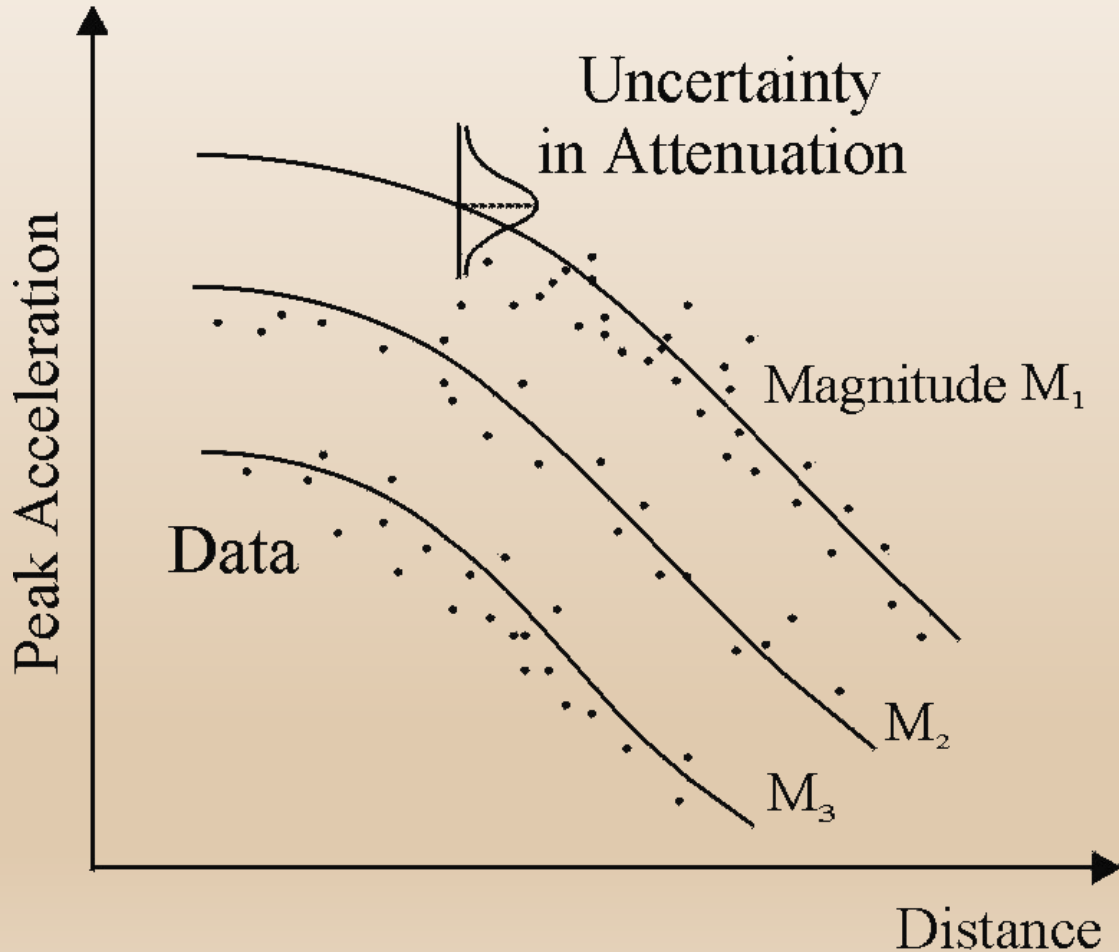
Site Class
Basin Effects
Ridgetop Effects

Site Class	Generalized Description	V_s^{100} (ft/s)	V_s^{30} (m/s)
A	Hard Rock	>5,000 ft/s	>1,520 m/s
B	Rock	2,500 to 5,000 ft/s	760 to 1520 m/s
C	Very Dense Soil/Soft Rock	1,200 to 2,500 ft/s	360 to 760 m/s
D	Stiff Soil	600 to 1,200 ft/s	180 to 360 m/s
E	Soft Clay Soil	<600 ft/s	<180 m/s
F	Requires Site Response Analysis		

Ground Motion Analysis: Basic Components

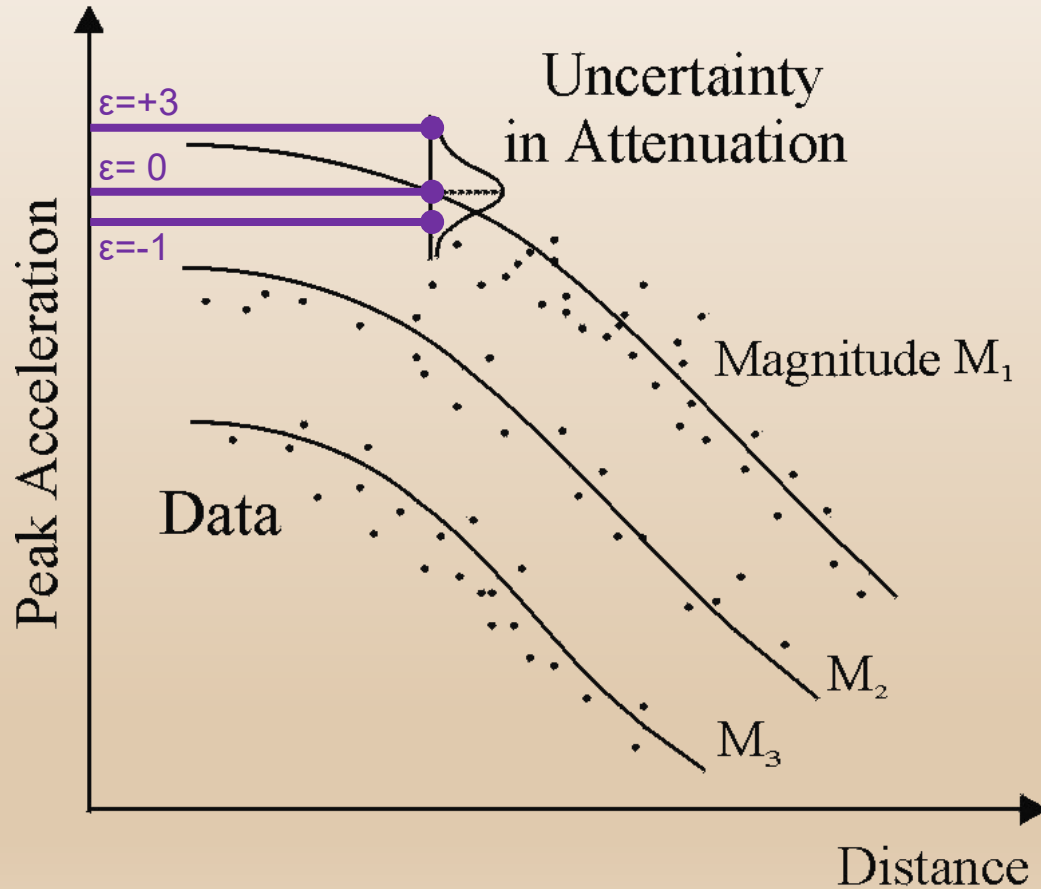


Ground Motion Prediction Equations (GMPEs)

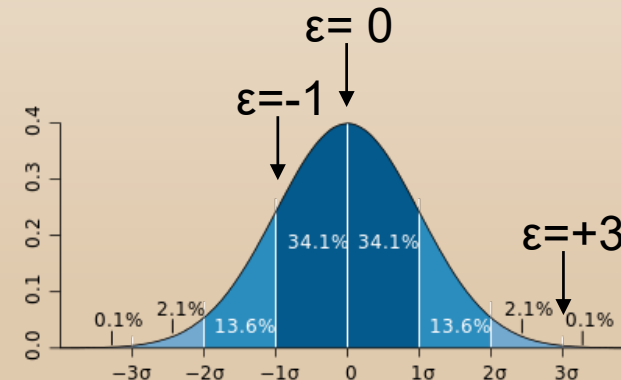


Ground Motion Analysis: Basic Components

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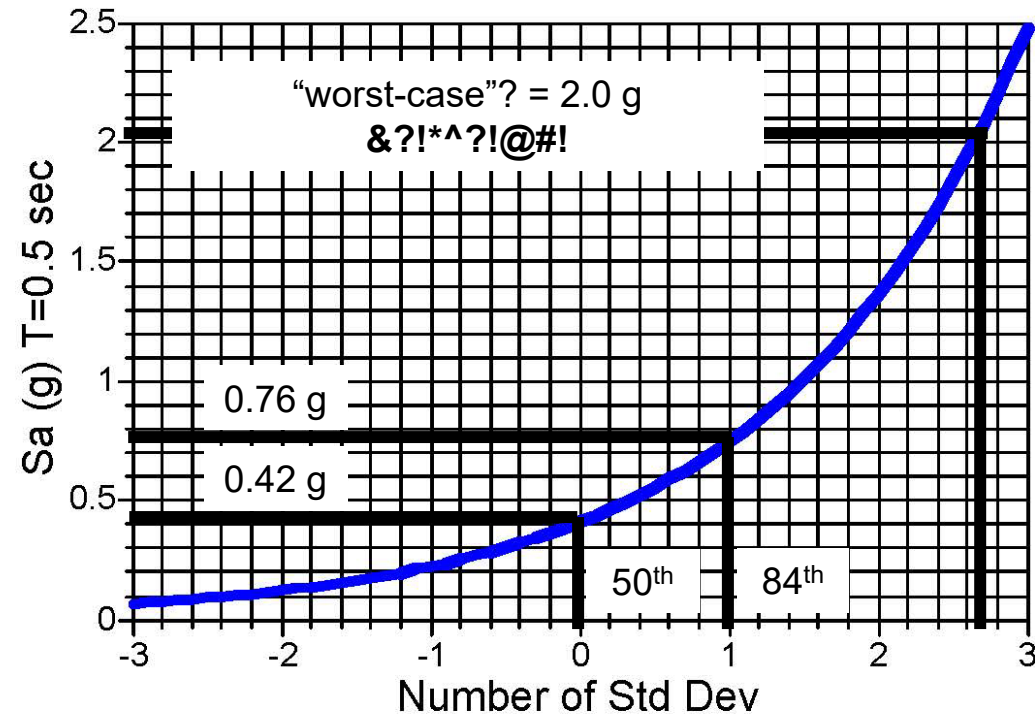
Given GM variability, what ground motion should be selected as a deterministic maximum?



Deterministic Approach: Variability in Ground Motion Prediction

Common practice:
Select 50th(median)
or 84th-percentile
ground motion level

But “worst-case”
ground motion will
exceed these
selected values



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- Key Guidance Documents

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- “Worst-Case” vs. “Reasonable”

Probabilistic Seismic Hazard Analysis (PSHA)

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Probabilistic Seismic Hazard Analysis (PSHA)

Considers all possible EQs and all possible ground motion levels

- Computes rates for each EQ/GM scenario
- Ranks scenarios in order of decreasing severity of shaking, using specified spectral acceleration
- Sums all rates of scenarios having ground motions above a specific level

Results in site-specific seismic hazard curve

Form of Hazard Calculation, with explicit treatment of ground motion aleatory variability (M, R, ϵ):

$$v(Sa > z) = \sum_{i=1}^{nSource} N_i(M_{\min}) \int \int \int_{MR\epsilon} f_{mi}(M) f_{Ri}(r, M) f_{\epsilon}(\epsilon) P(Sa > z | m, R, \epsilon) d\epsilon dR dM$$



Seismic Source Characterization

Areal Source Zones (Background Seismicity)

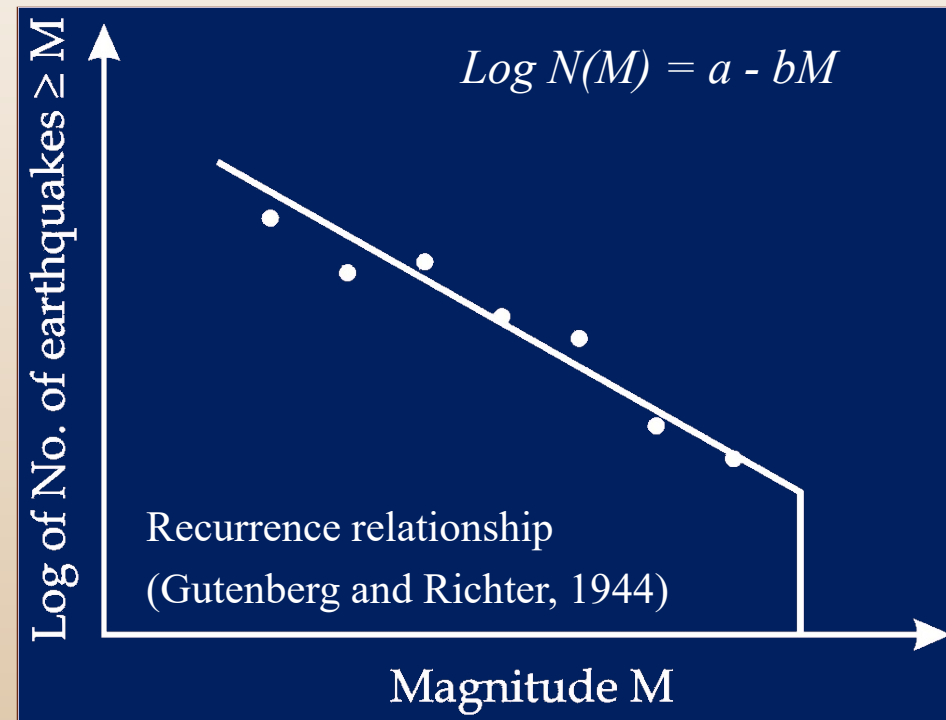
Rate of activity from historical and instrumental seismicity catalogs

- Within seismotectonic zones, assume spatial homogeneity for specified grid size
- Calculate a - and b -values in G-R relationship for each grid cell

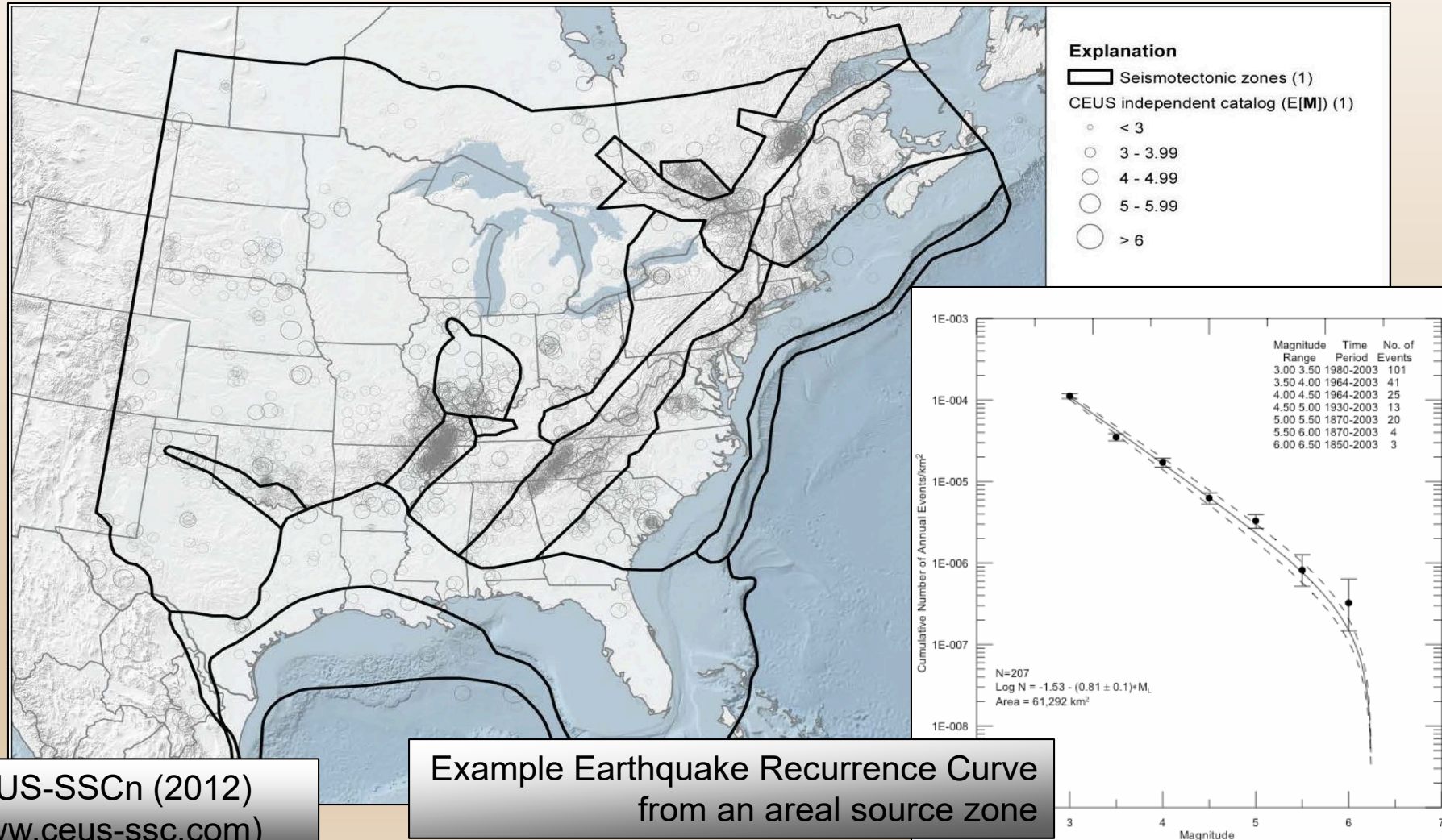
Accounts for earthquakes on unidentified faults

Maximum magnitude

- Western US usually assume $M_{mx} \sim 6.5$
- Central_Eastern US assume $M_{mx} \sim 8$



Regional Seismotectonic Zones (example: Central and Eastern US)



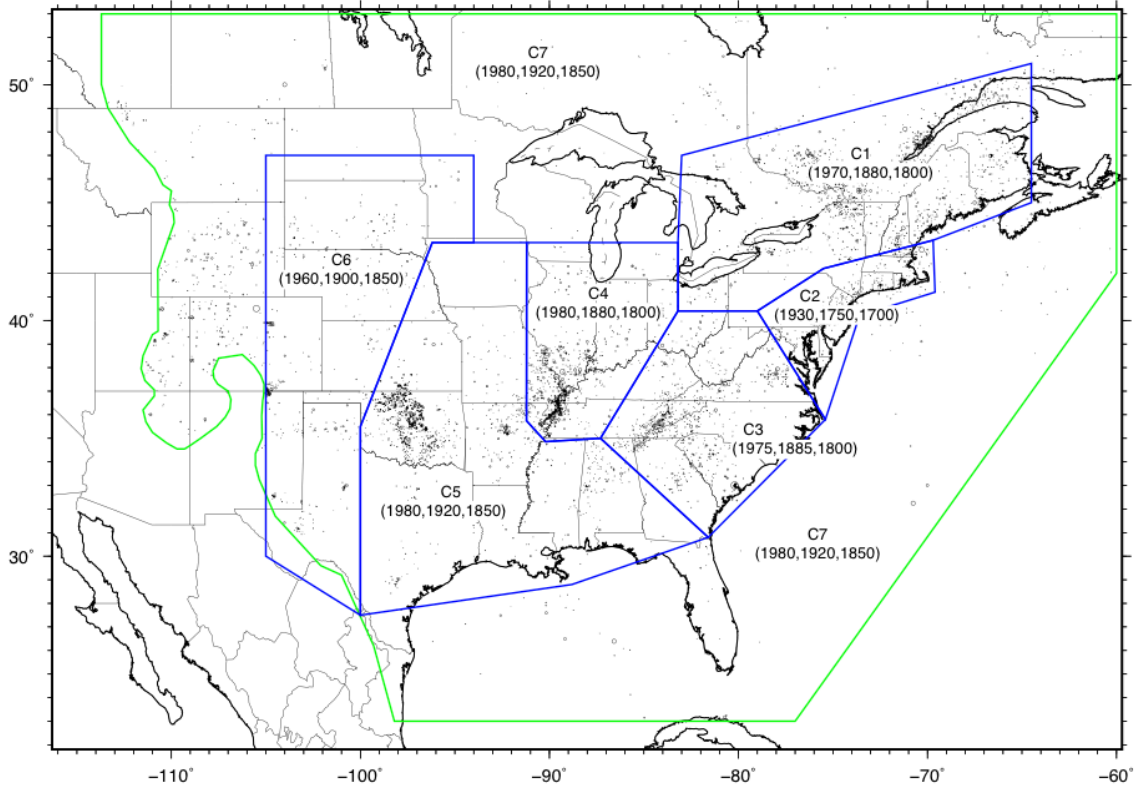
CEUS-SSCn (2012)
(www.ceus-ssc.com)

Example Earthquake Recurrence Curve
from an areal source zone

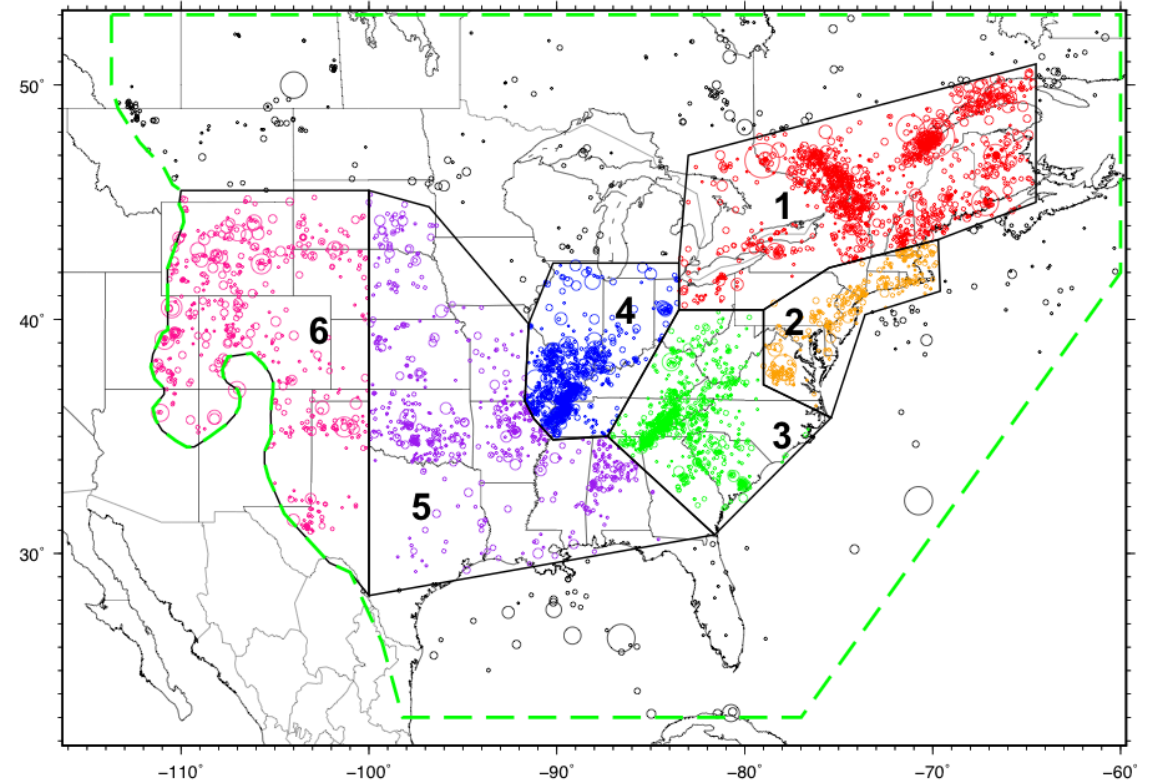


Regional Seismotectonic Zones (example: Central and Eastern US)

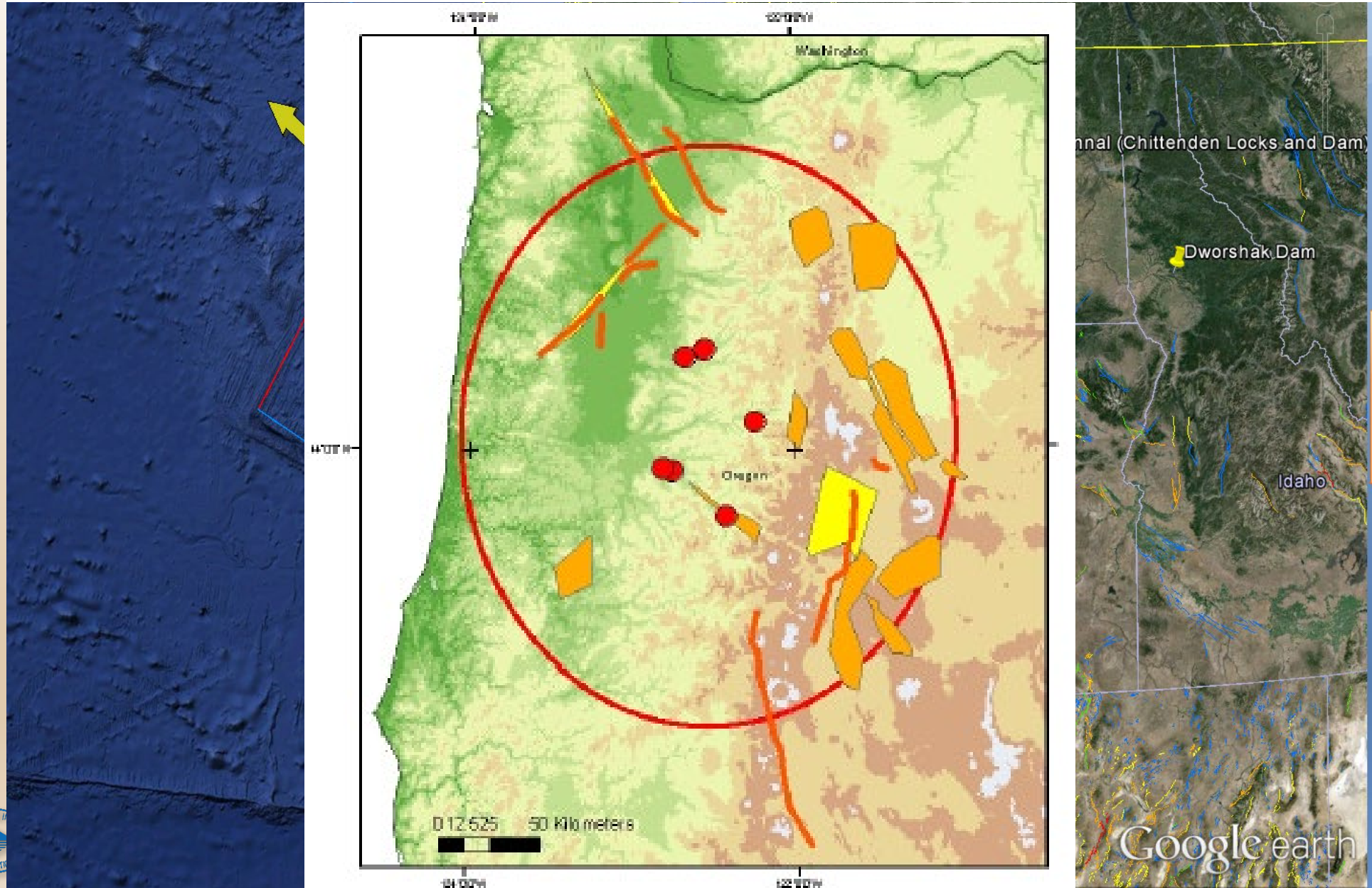
2014 NSHM CEUS catalog zones



2018 NSHM CEUS catalog zones



Seismic Source Model: Faults



Cascadia Subduction Zone Seismic Sources

Ductile Zone
Intraslab Earthquakes
 M_w from ~5.0-7.5

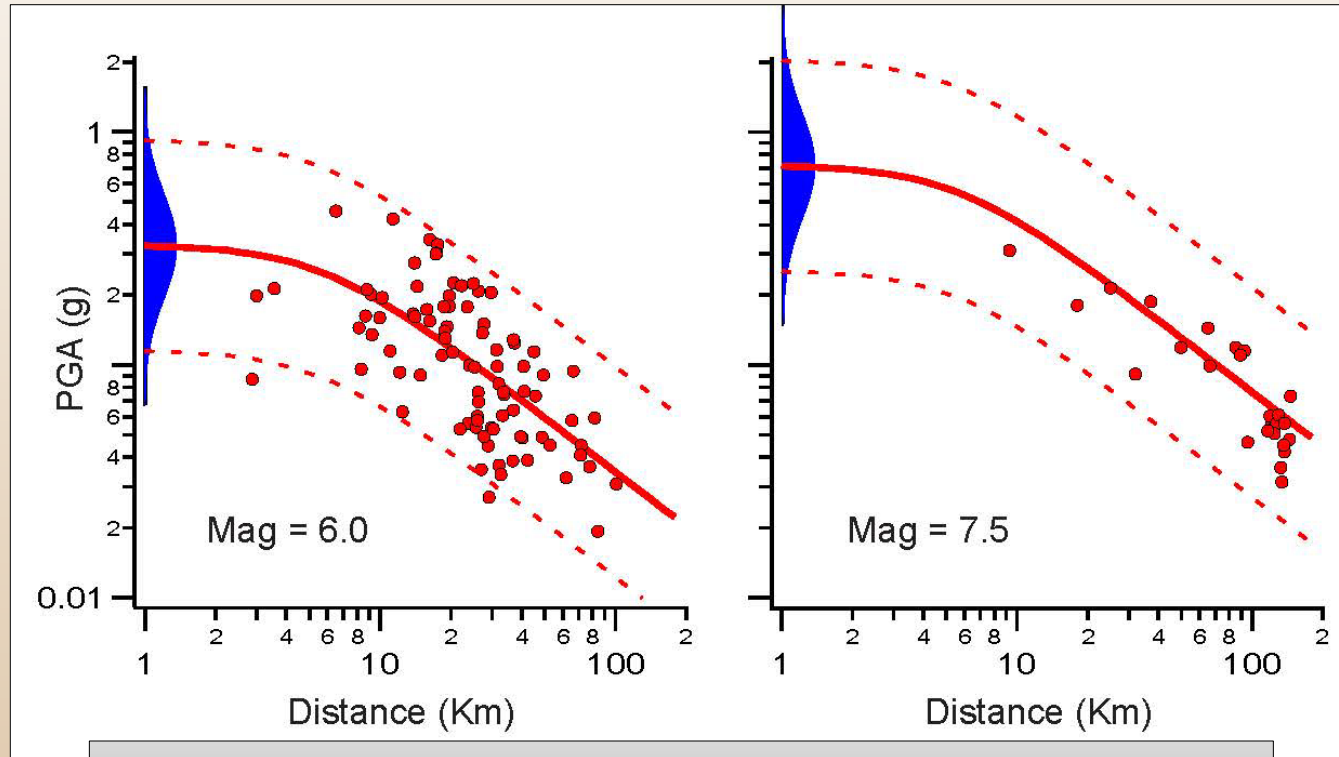
Locked Zone
Interface Earthquakes
 M_w from ~8.0-9.2

Crustal Source
Earthquakes
 M_w from ~5.0-7.0



PSHA: Ground Motion Prediction Equations (GMPE)

Examples of GMPEs for two magnitudes, with schematic uncertainties



Typical current practice uses “ergodic” models:

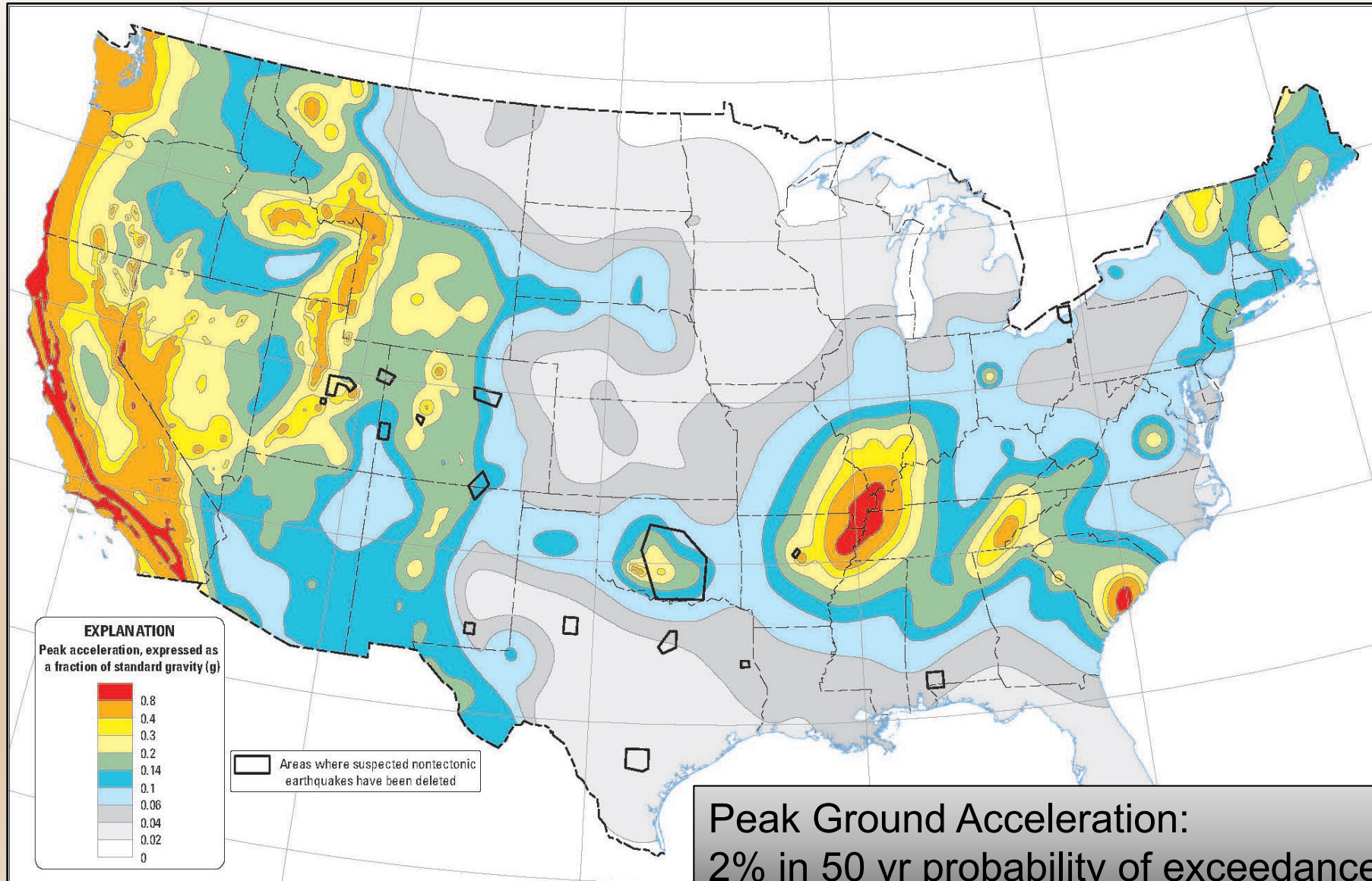
- USGS (2014) for screening and regional analyses
- NGA-West2 or NGA-East for site-specific analyses

Research moving toward “non-ergodic” models

Estimating Expected Strong Ground Motions using worldwide empirical databases
“Next Generation Attenuation” equations

- Western US: “NGA-West 2”
- Central + eastern US: “NGA-East” (Sept 2017?)
- Cascadia Subduction Zone: “NGA-Cascadia” (2018?)

USGS (2014) National Seismic Hazard Mapping Project

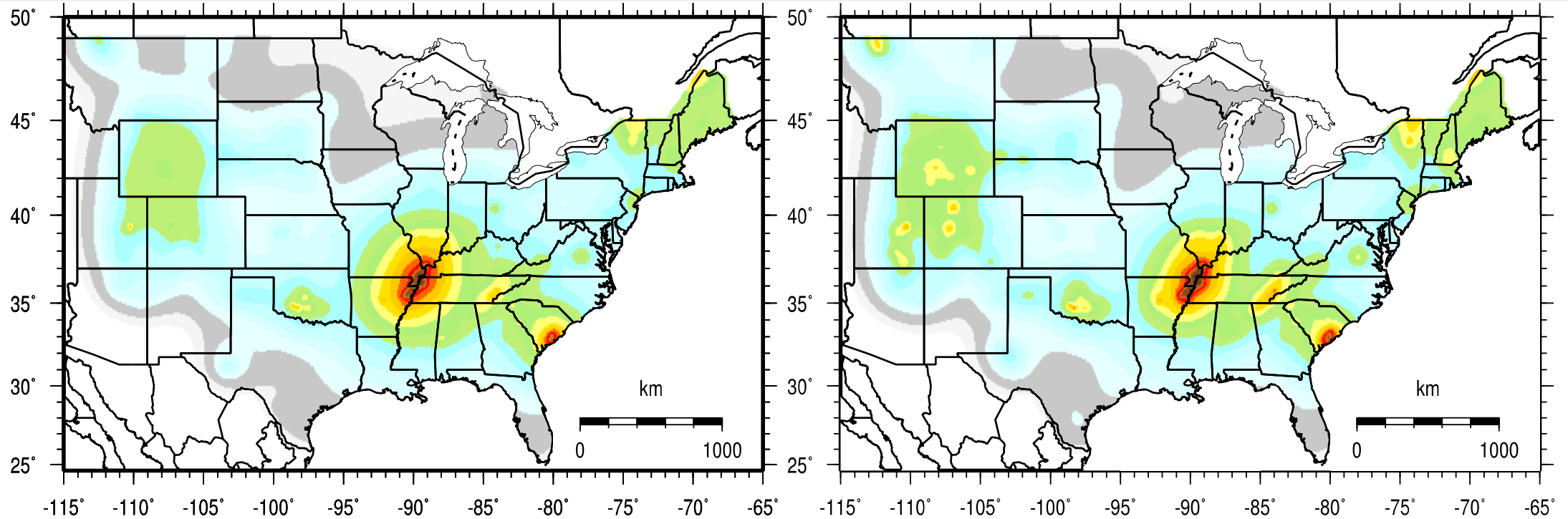


Peak Ground Acceleration:
2% in 50 yr probability of exceedance



USGS (2018) – PRELIMINARY RESULTS

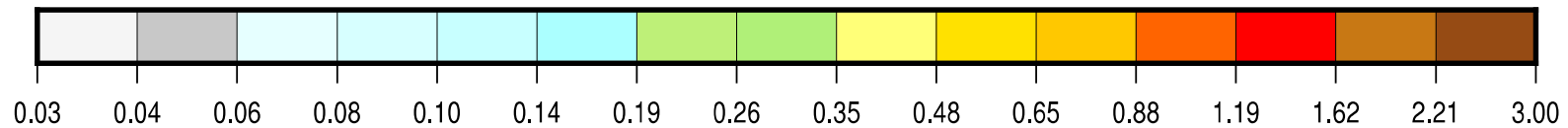
National Seismic Hazard Mapping Project



2018 NSHM CEUS GMMs (map 1)

2014 NSHM CEUS GMMs (map 2)

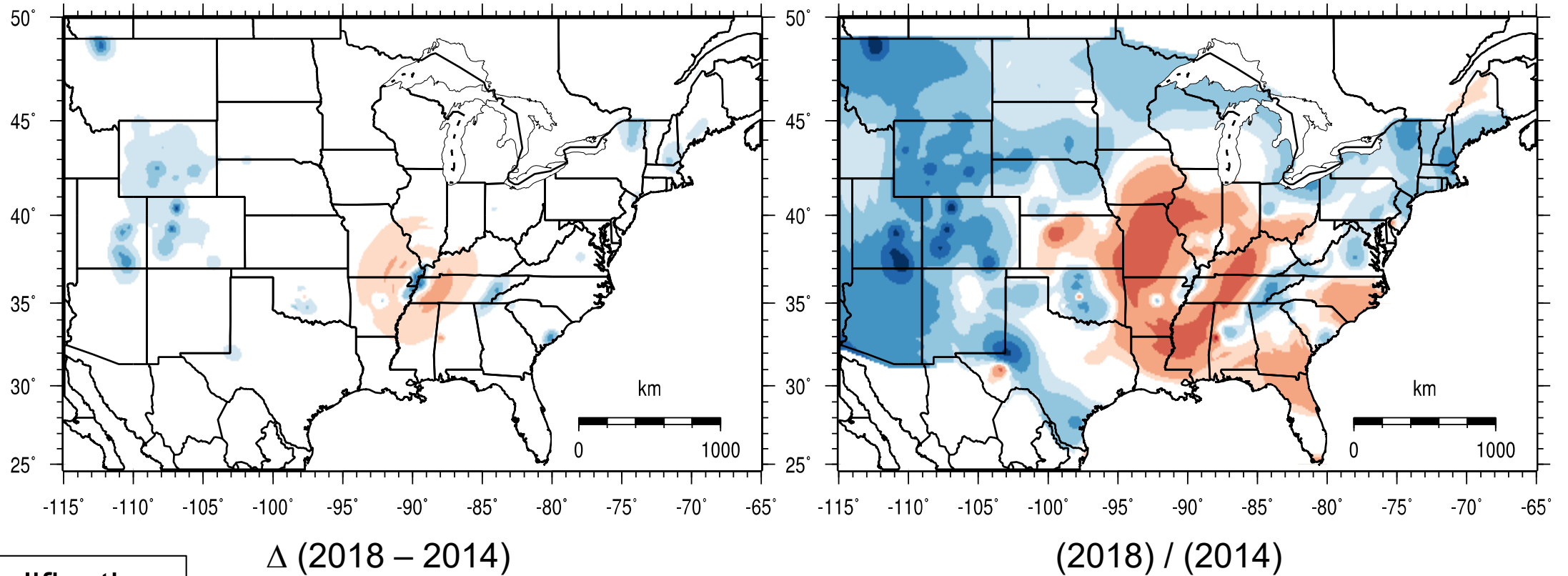
CEUS Amplification:
New models (linear
and non-linear)



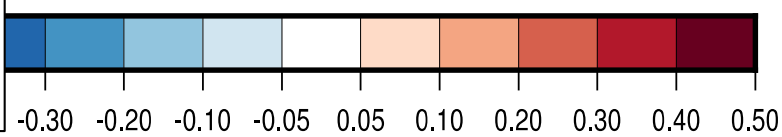
0.2 Second Spectral Acceleration (g)

USGS (2018) – PRELIMINARY RESULTS

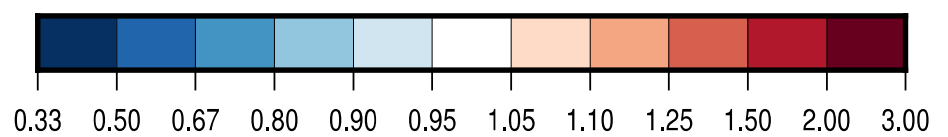
National Seismic Hazard Mapping Project



CEUS Amplification:
New models (linear
and non-linear)



Difference



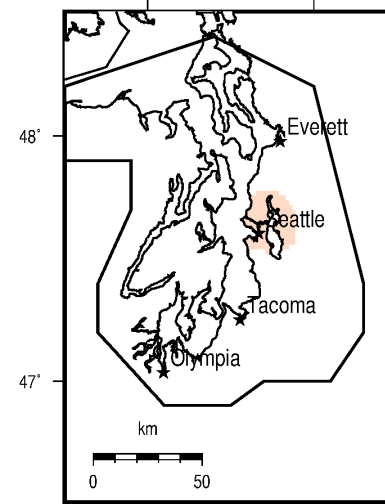
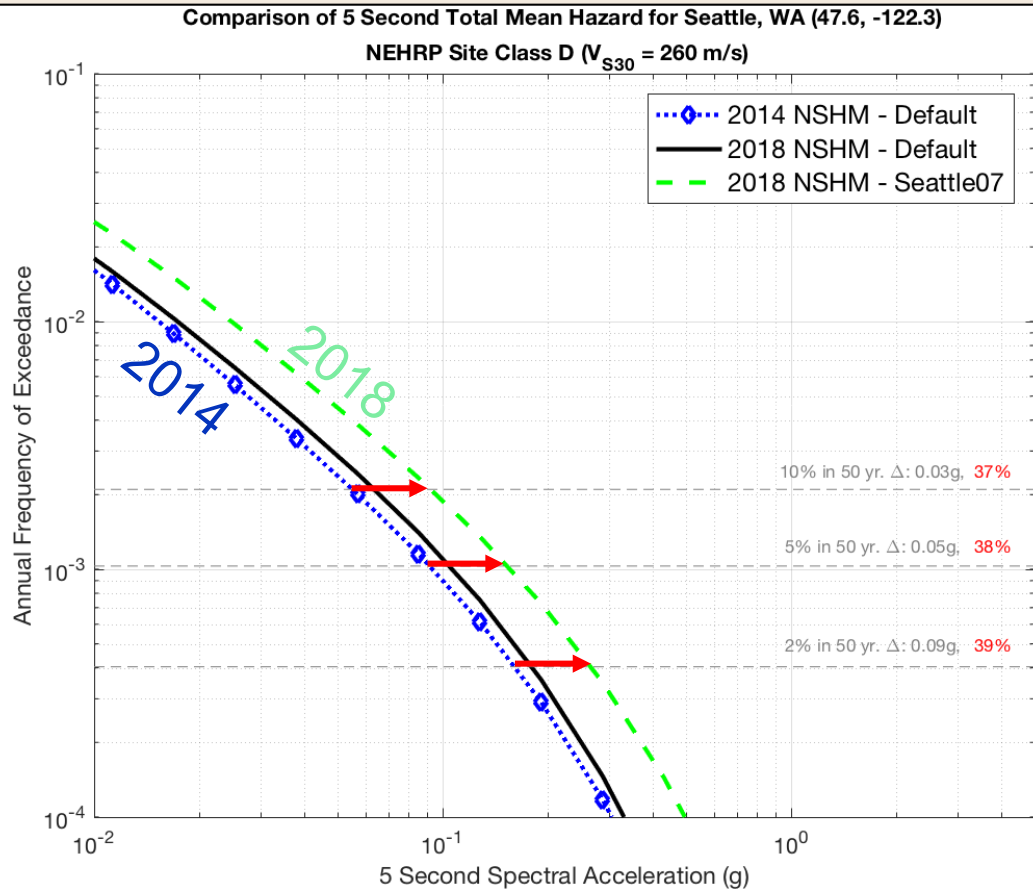
Ratio



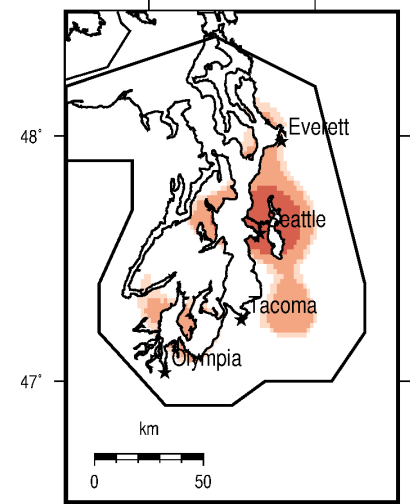
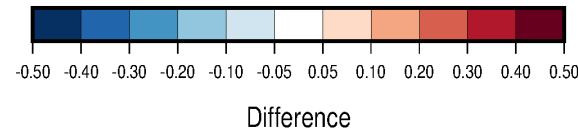
USGS (2018) – PRELIMINARY RESULTS

National Seismic Hazard Mapping Project

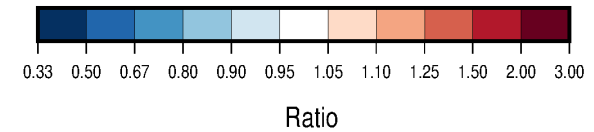
WUS Amplification:
Basin models for
SF, LA, UT, WA



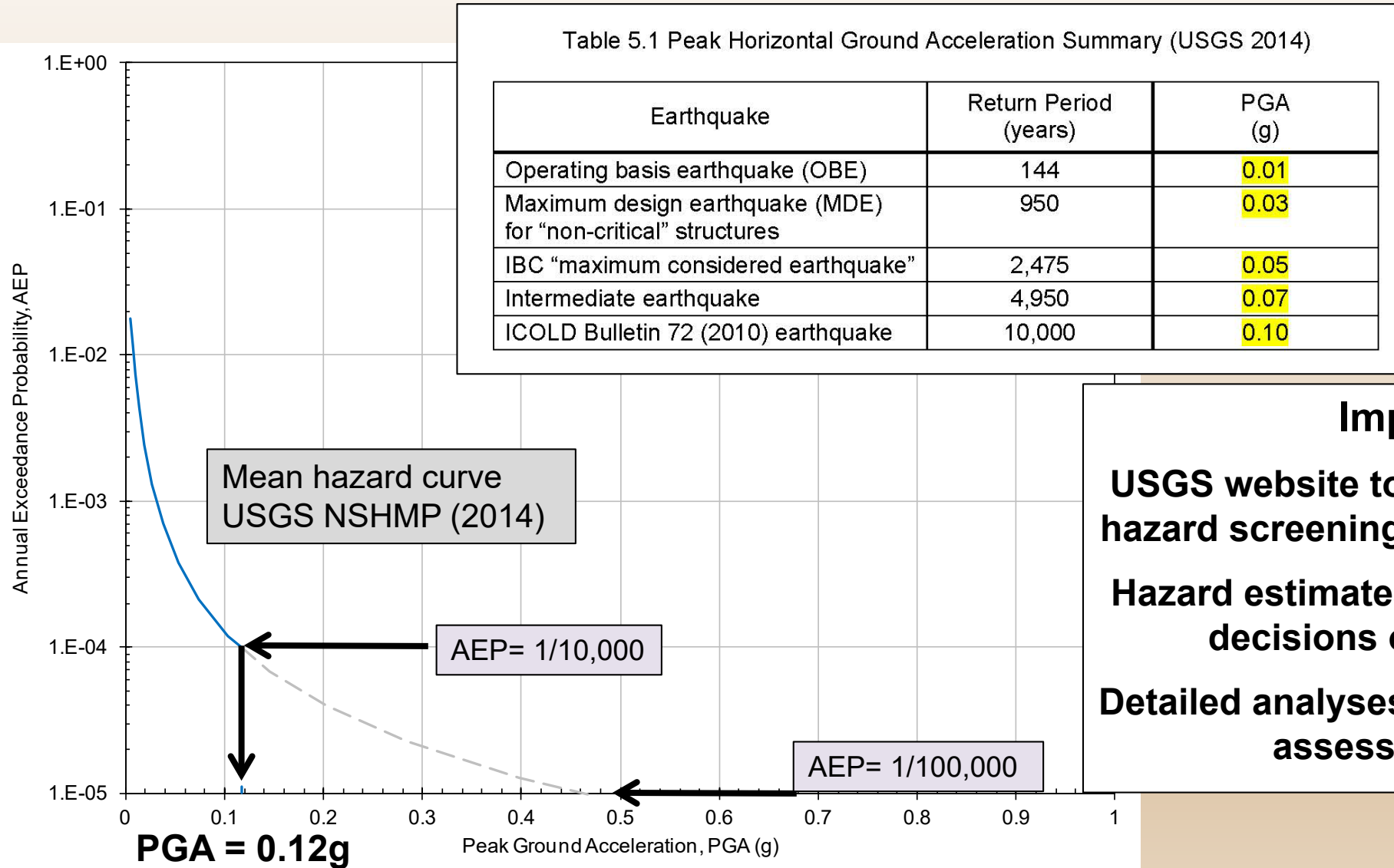
Δ (2018 - 2014)



(2018) / (2014)



USGS Seismic Hazard Website Tools



Important Note:

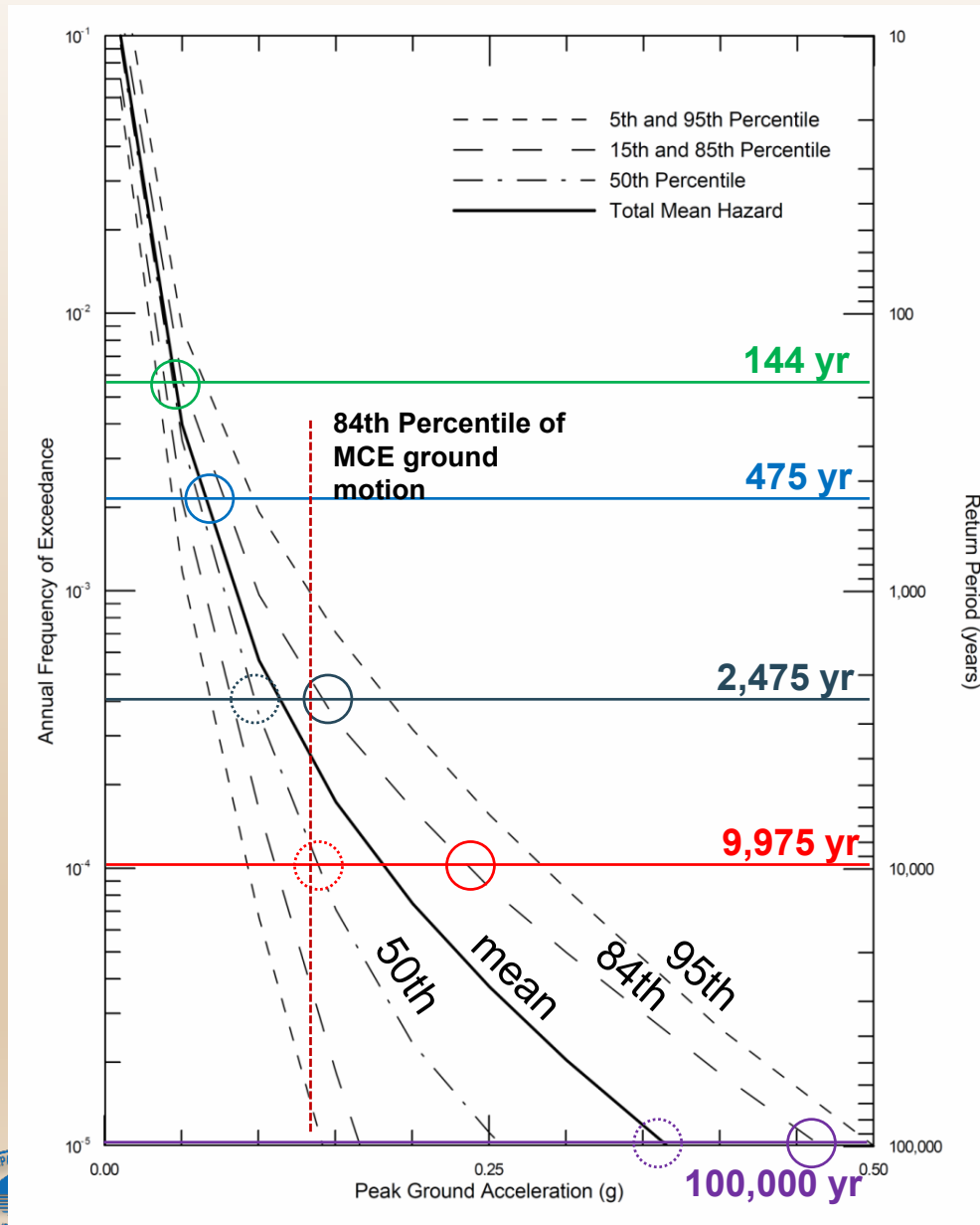
USGS website tools acceptable for regional hazard screening and Periodic Assessments

Hazard estimates not acceptable for critical decisions or engineering design

Detailed analyses required for high-level risk assessments and design



Site-Specific Seismic Hazard Curve



Site-Specific Probabilistic Seismic Hazard Analysis (PSHA)

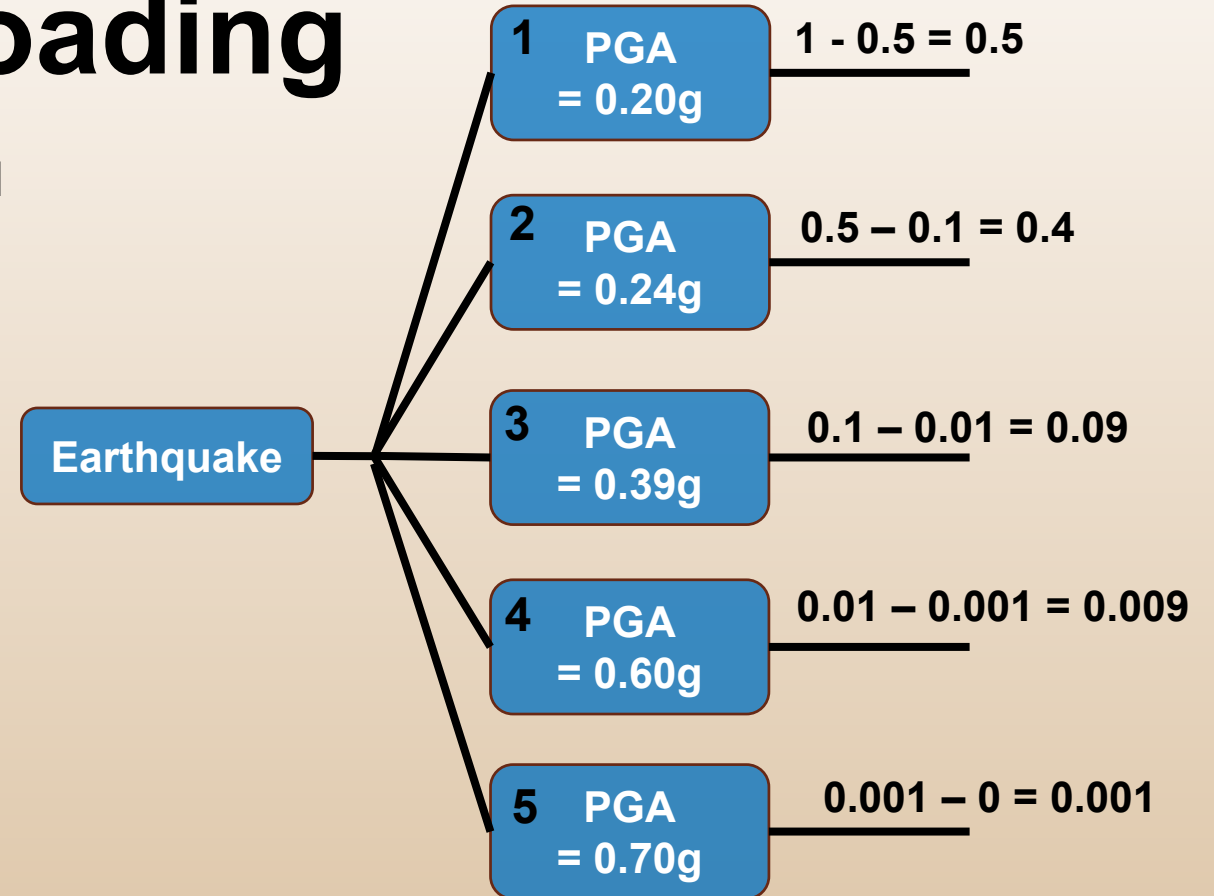
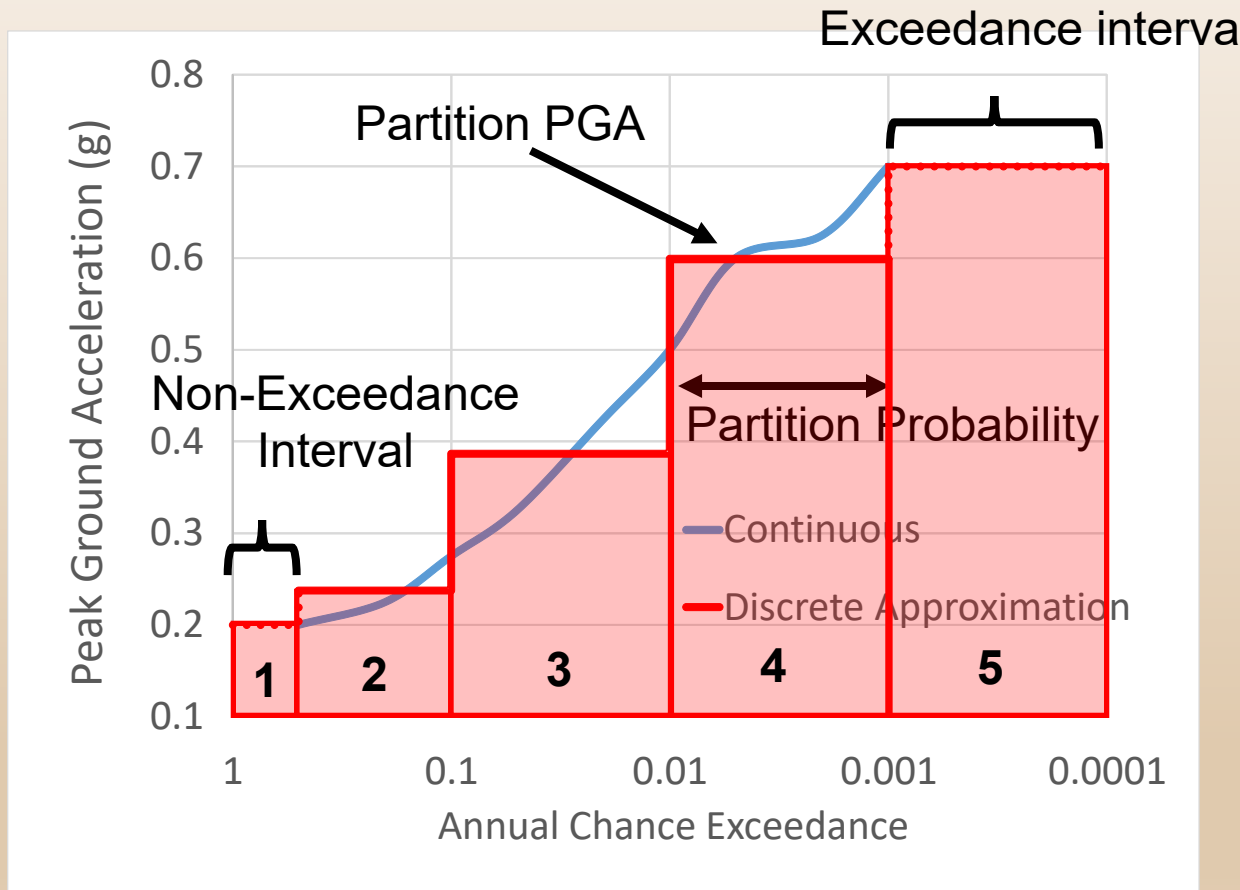
For design

- Operating Basis EQ (OBE, 144 yr)
- MCE GM (50th or 84th percentile)

For risk assessment

- 2,475 yr GM (AEP=4xE-04)
- 9,975 yr GM (AEP=1xE-04)
- 100,000 yr GM (AEP=1xE-05)
(if GMPE allow)
- 50th or 84th percentile

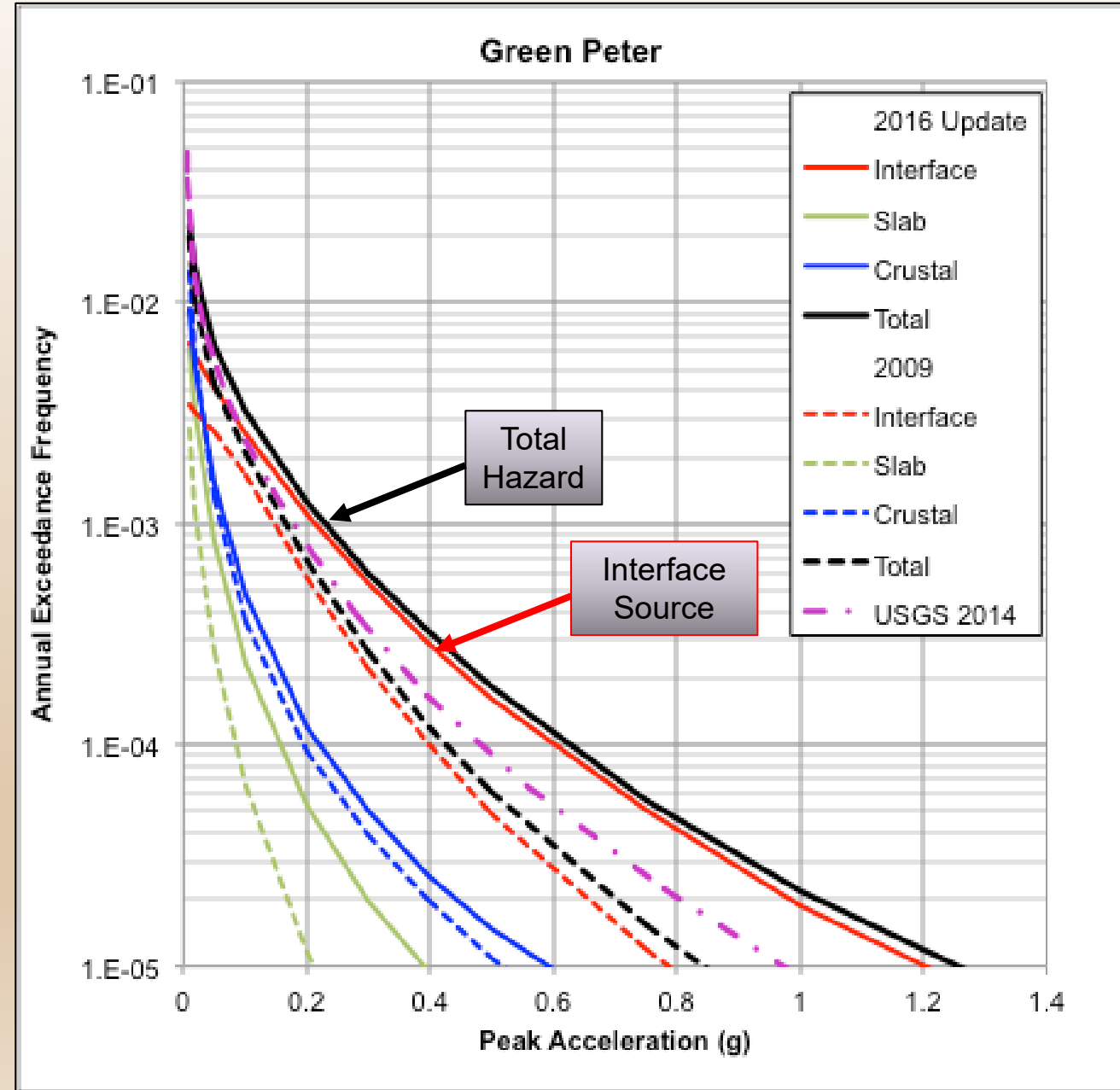
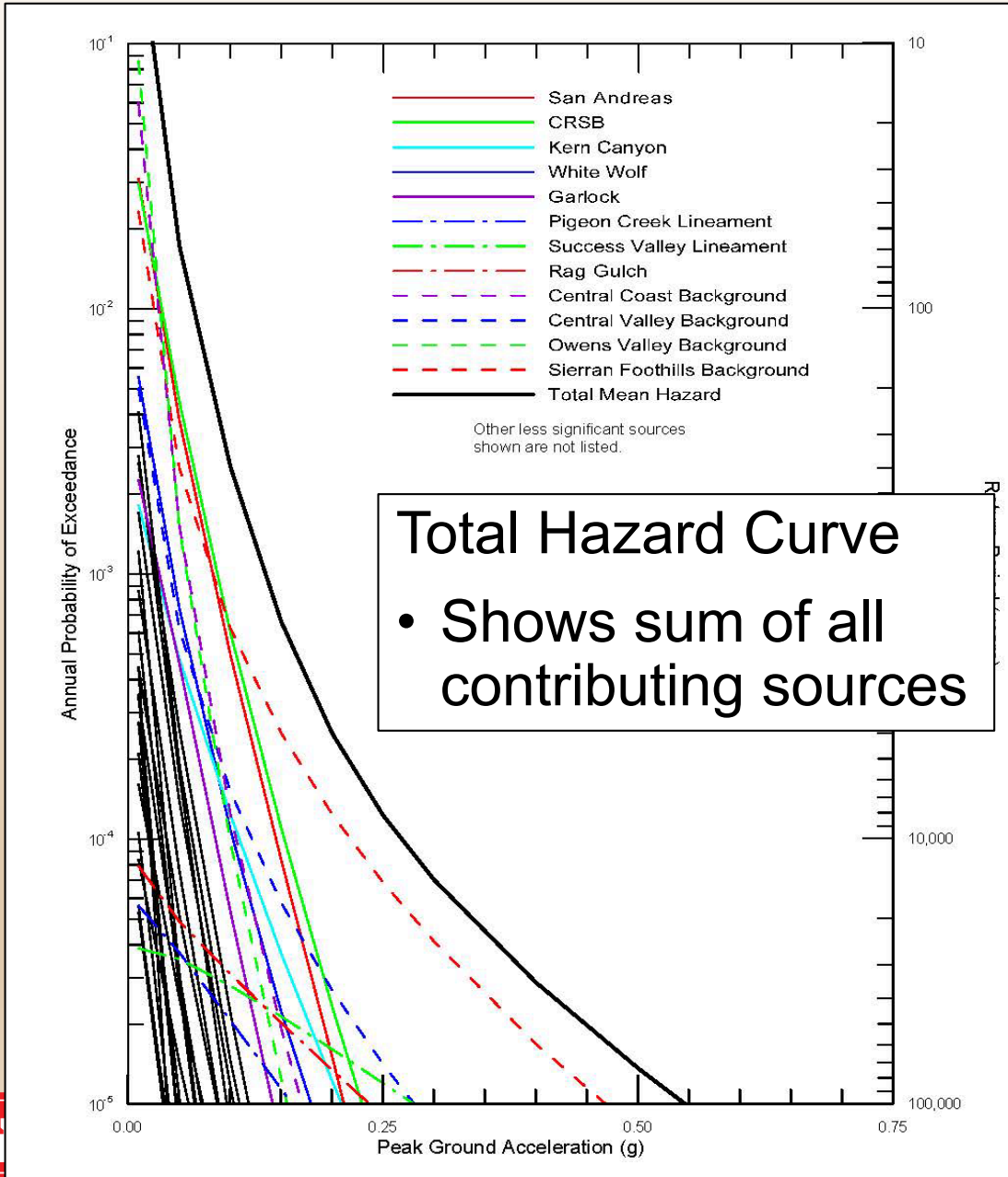
Example: Seismic Loading



$$\sum (\text{partition probabilities}) = 1$$

These partitions are mutually exclusive

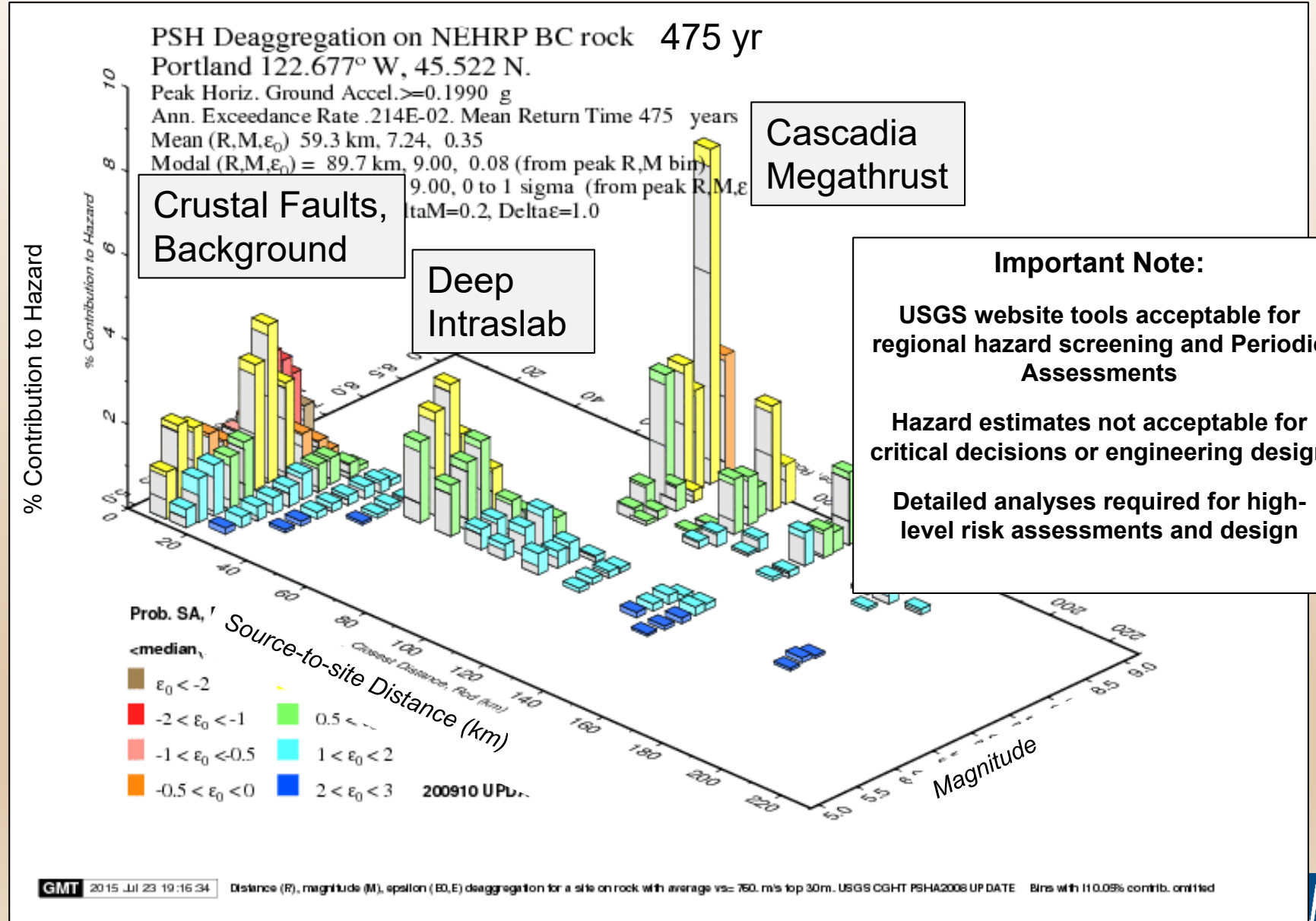
Site-Specific Seismic Hazard Curve: Fault Contributions



Site-Specific Seismic Hazard Curve: Fault Contributions

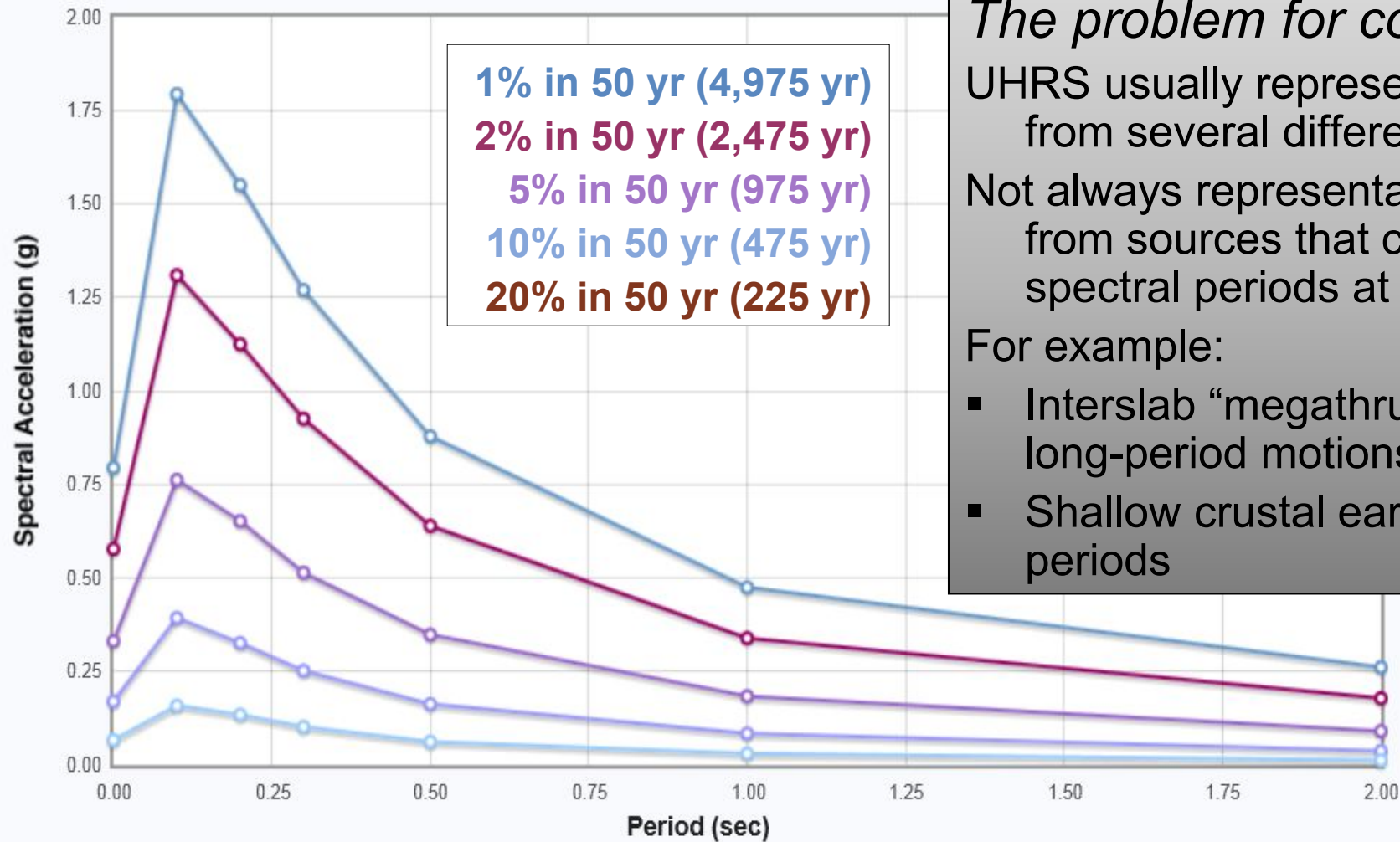
De-aggregation Plot

- Shows distance and magnitude characteristics of specific sources
- Identify primary contributors to hazard
- Use for selecting historical GM records while developing GM time histories



Uniform Hazard Response Spectra

Latitude: 36.92851 Longitude: -90.28381



The problem for complex structural analyses:

UHRS usually represent expected ground motions from several different seismic sources

Not always representative of spectral accelerations from sources that contribute most to certain spectral periods at a site

For example:

- Interslab “megathrust” earthquakes may dominate long-period motions
- Shallow crustal earthquakes may dominate short periods

<http://geohazards.usgs.gov/hazardtool/application.php>



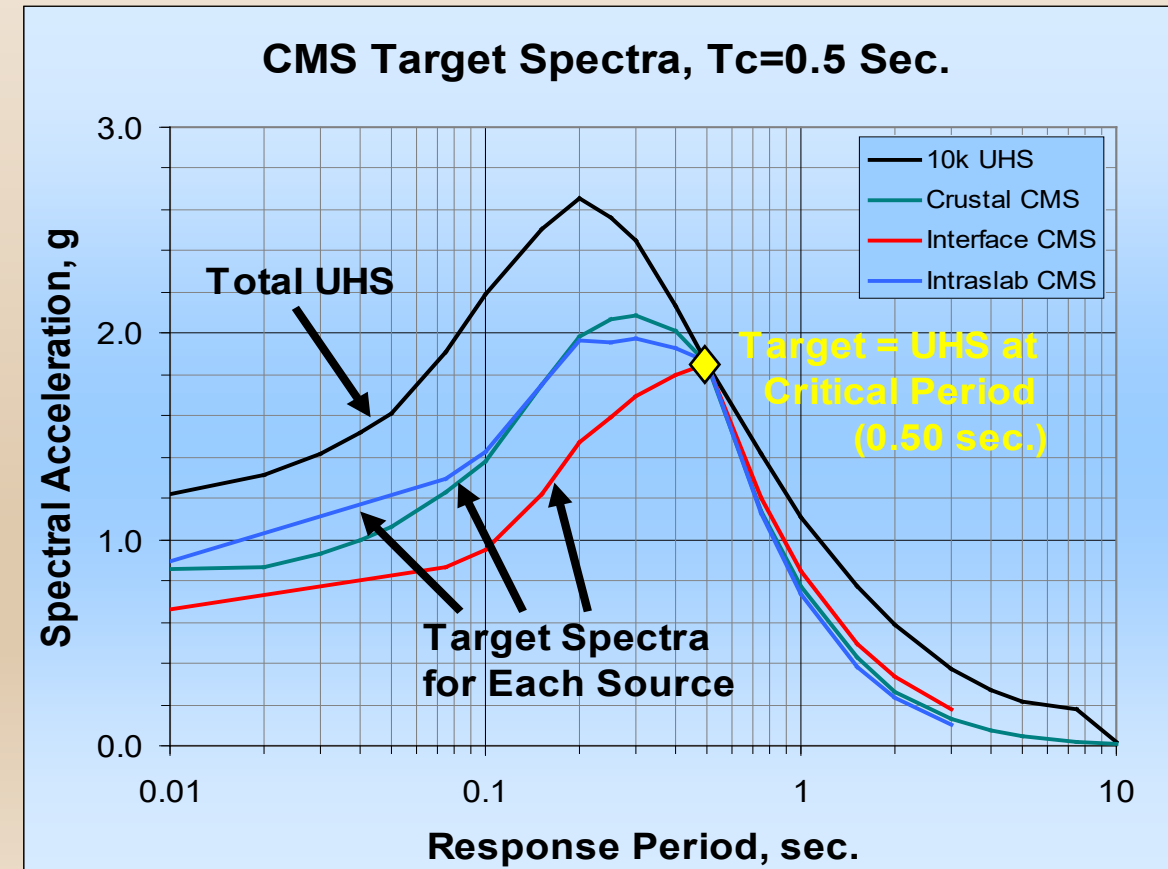
Conditional Mean Spectra (CMS)

“Composite” response spectrum obtained from multiple source-specific EQs

- Conditioned on peak response for a specified target spectral period (i.e., 0.3 sec)
- Target spectral period: typically chosen as the critical period of a structure

The sticky parts:

- Critical periods for structures rarely known
- Structures may have nonlinear behavior
- Many potential critical periods should be considered
- Central range of response periods may be best depicted by UHRS
- CMS can also be applied outside this central range of response periods



Ground Motion Time Histories

Using only PGA from PSHA neglects timing and duration of strong shaking

Instead, use ground motion records from actual EQs

- Similar to expected EQ at the site
- De-aggregate hazard to understand main contributors and types of EQs

“Scale” the history of expected ground motion to fit site hazard

- Scaling vs. Spectral Matching
- Select several historical earthquake records with comparable magnitude, shaking duration, fault type, basin effects, etc.

Suites of time histories should be developed for multiple return periods

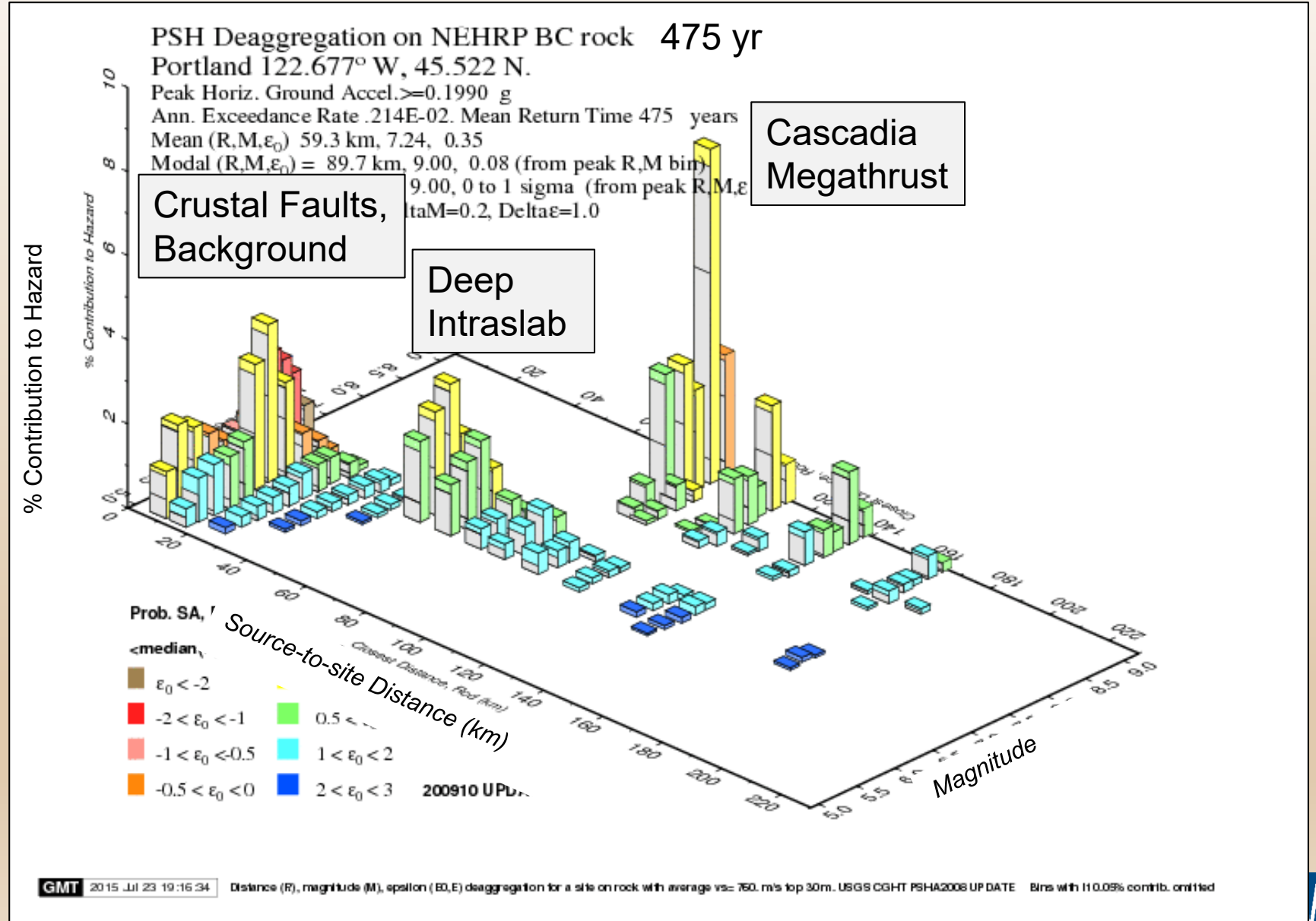
Needed for dynamic stability analyses (*FLAC*, *SHAKE*, or *LS-DYNA*)



Ground Motion Time Histories

De-aggregation Plot

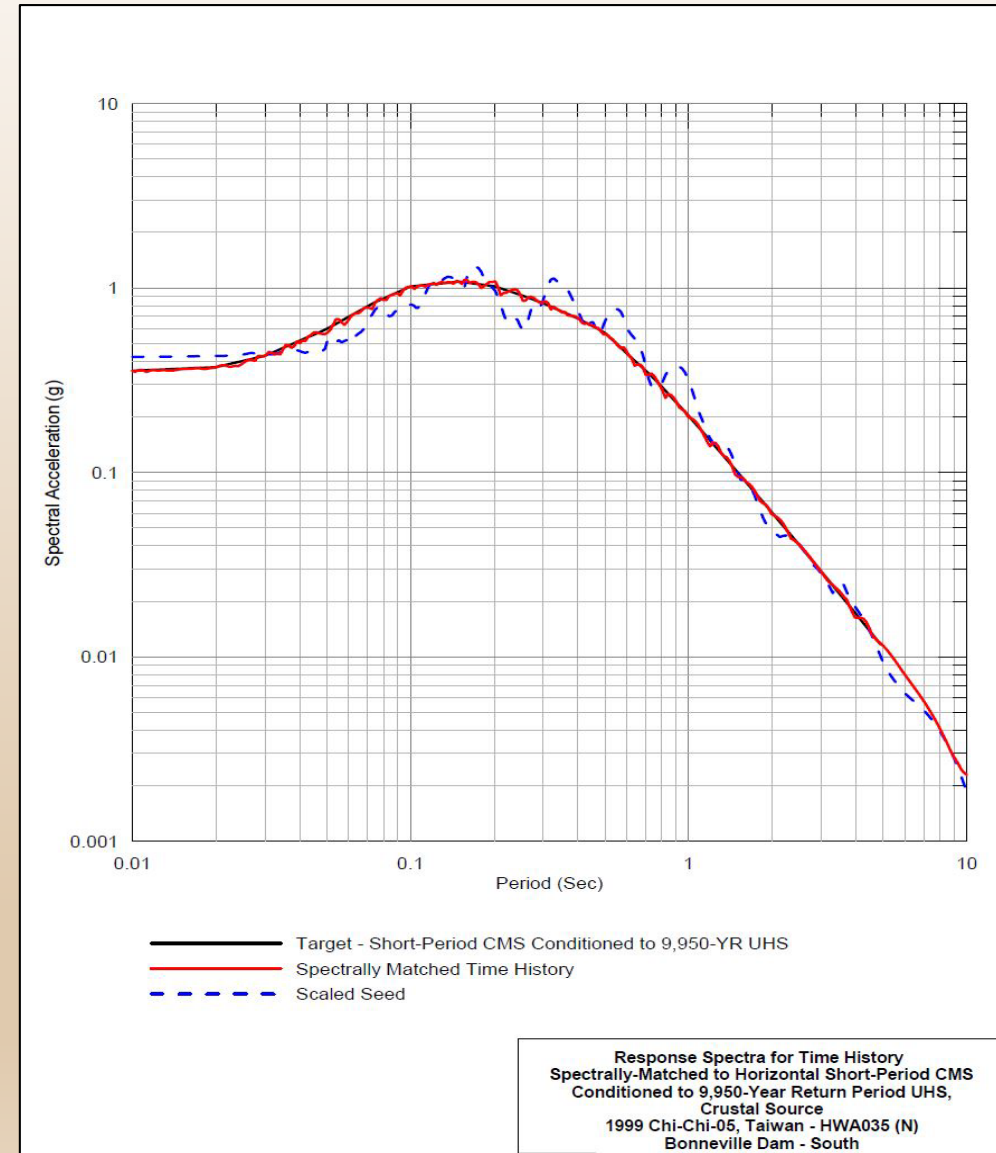
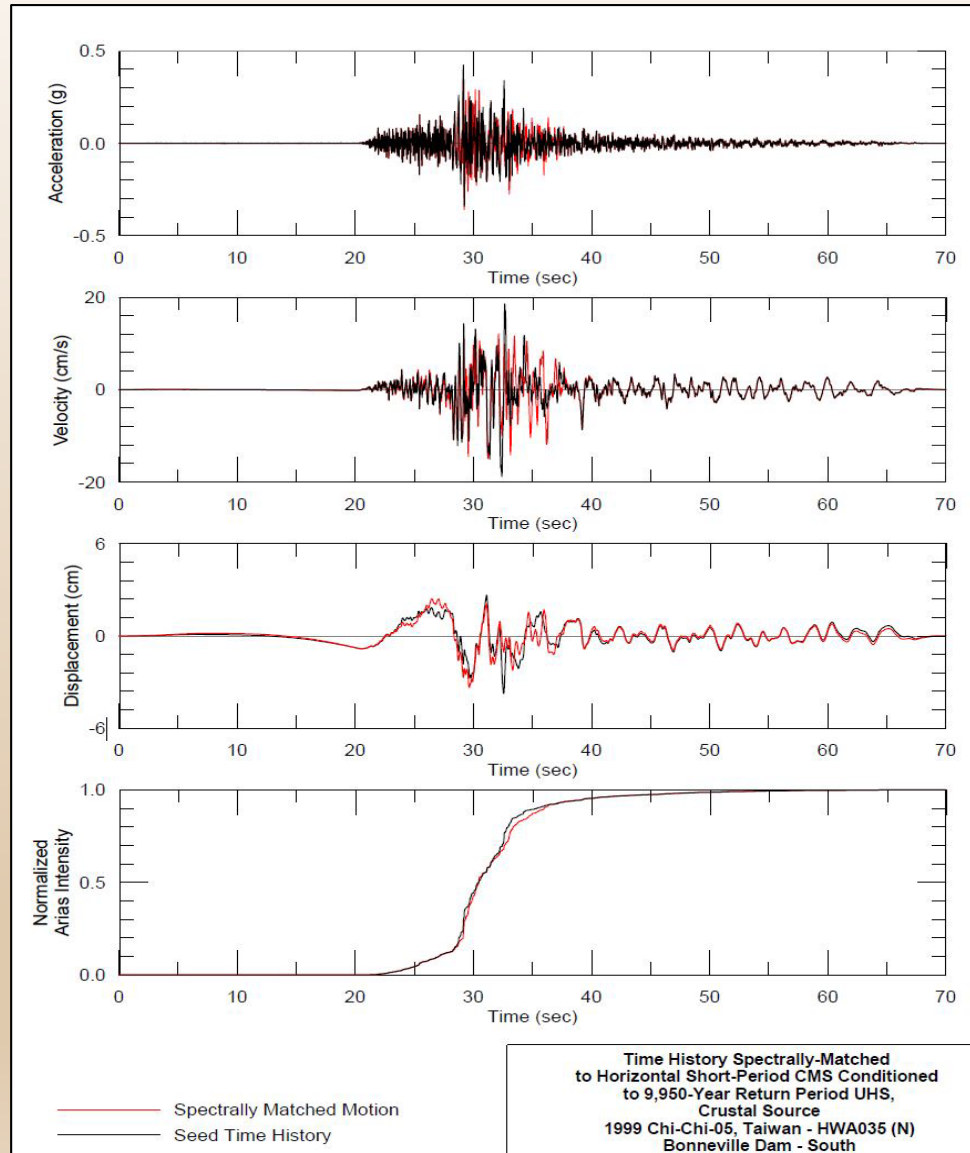
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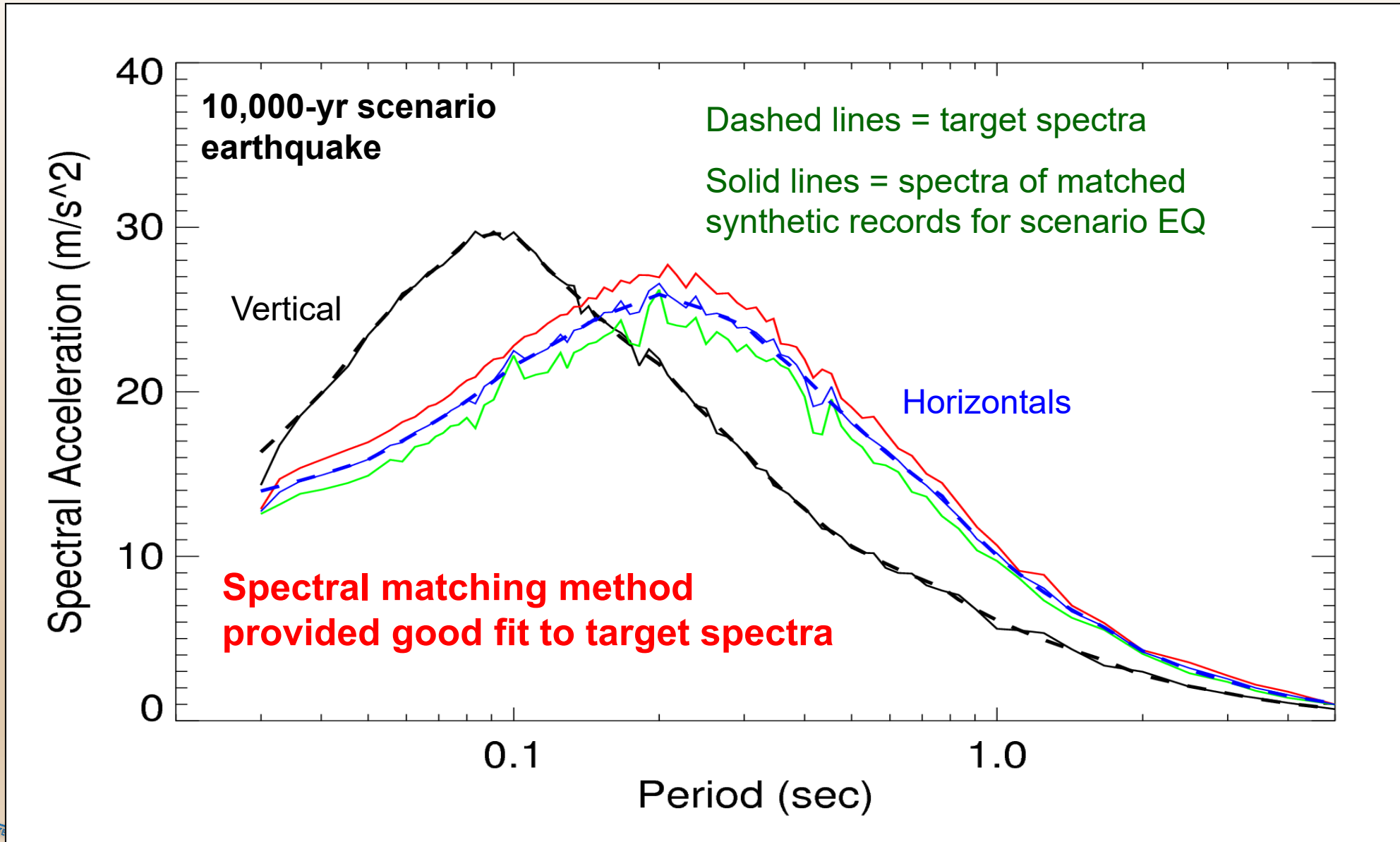
Ground Motion “Seed” Time Histories

For comparable hazard-driving seismic source:

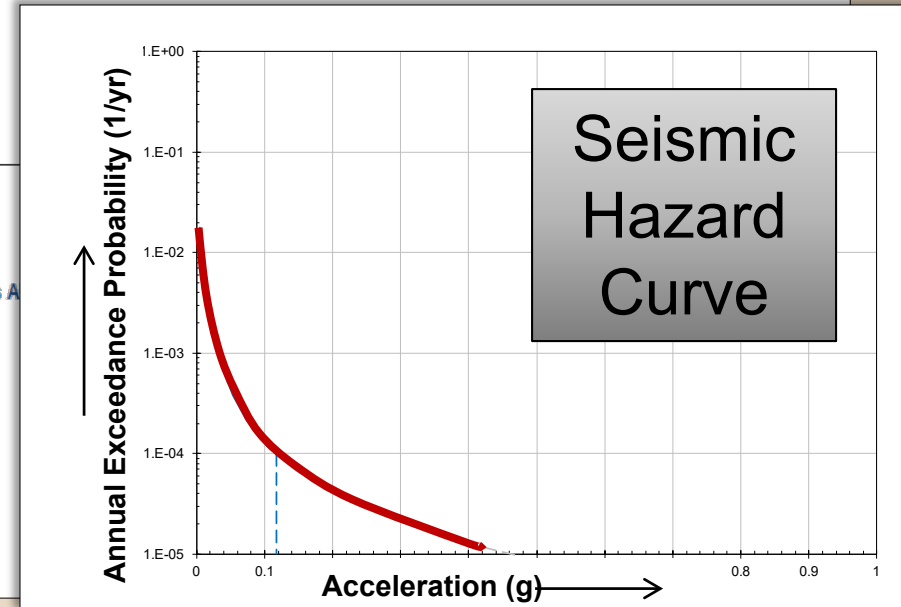
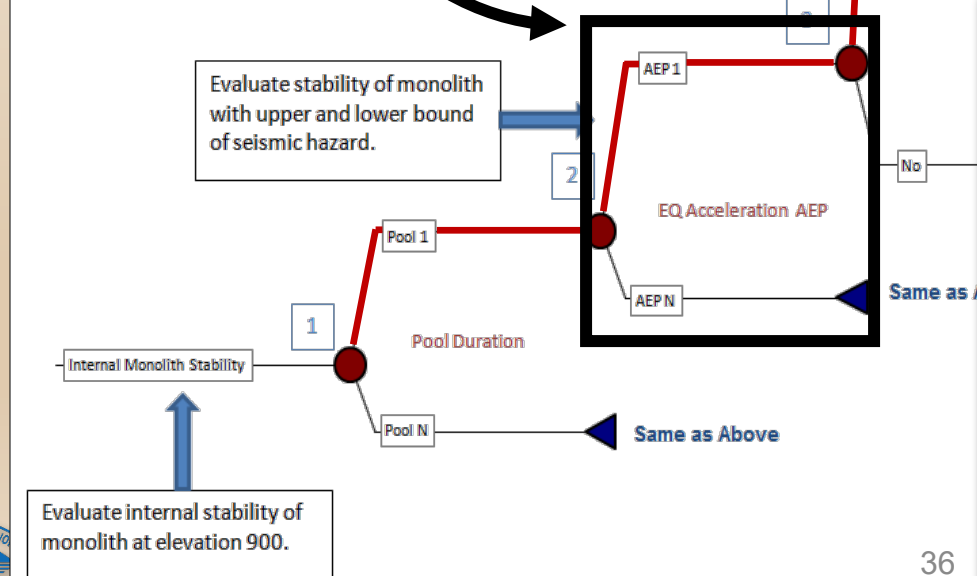
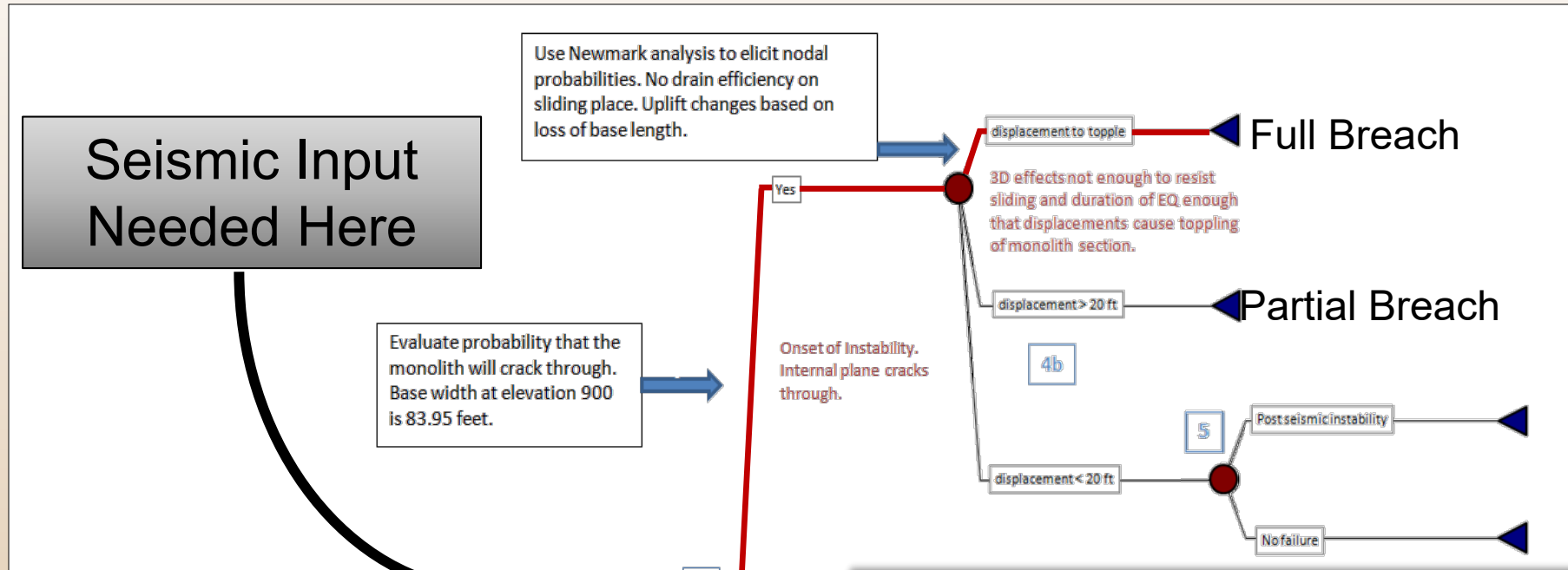
- Similar source type, magnitude, distance, and site conditions
- Identify reliable historical GM record(s)
- Scale or match historical record to target spectrum



Ground Motion Time Histories



Seismic Loading in Risk Assessments: PFMA Event Tree



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Thank You